VIRGINIA AMERICAN WATER DEVELOPER INSTALL WATER MAIN AND SERVICE LINE SPECIFICATIONS AND DETAILS

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VIRGINIA AMERICAN WATER DEVELOPER INSTALL WATER MAIN AND SERVICE LINE SPECIFICATIONS AND DETAILS

GENERAL NOTES

1. "Owner" used in these specification and details means "American Water".

2. "Engineer" used in these specification and details means "American Water Engineering Department".

3. "Contractor" used in these specification and details means "Developer and its contractor".

4. Developer is not permitted to make any connection to existing Water Company Facilities. The Water Company or its designated Contractor shall make all such connections to provide water service to the main extension. The Water Company or its designated contractor shall perform all main installation within the existing public right-of-way.

5. Developer shall provide cut sheets sealed by a state VA licensed professional Surveyor/Engineer for all water (and Dale City Sanitary Sewer) facility installations. The professional Surveyor/Engineer who seals and signs the cut sheets shall also provide the following statement on all sets: "The professional seal and signature appearing on this document certifies that information shown conforms to the approved Project Plan and/or actual field conditions."

6. Before construction, developer's state licensed surveyor shall place line and grade stakes identifying dedicated easements, main, service connections, and other appurtenances to ensure the water and sanitary sewer system is constructed in accordance with the approved project plans.

7. Developer shall contact VAW to obtain all construction specifications and details, before construction start and/or order materials.

8. Developer needs to submit water main extension and/or replacement construction schedule, call out each water main section construction sequence, including pressure test length, disinfection method, water main flushing and bacteria sample collection timeline.

DISPOSAL OF CHLORINATED WATERS

Chlorinated water can be discharged from new construction, distribution system repairs, water main breaks, hydrant testing, flushing, and other water utility operations. Depending on the situation, discharged chlorinated waters can have dramatic effects on the environment (i.e. fish kill in receiving streams). Water utilities can use various dechlorination techniques when discharging chlorinated water in the environment. The dechlorination technique to be used depends on the following parameters: chlorine level of the discharged water, water volume (flow and duration), type of discharge, and receiving environment.

Type of chlorinated water releases

The releases can be classified as planned, unplanned and emergency releases. Planned releases result from operation and maintenance activities such as disinfection of mains, testing of hydrants and routine flushing of distribution system lines for maintenance. In general, planned releases should be dechlorinated. Unplanned releases occur from activities such as main breaks, leaks and overflows. Water main flushing in response to WQ complaints is also included in emergency releases of chlorinated waters.

Based on the level of residual disinfectant, the source can be classified as low-level residual (< 4 mg/L, drinking water) or high-level residual (super chlorinated water, > 4 mg/L). The volume (flow & duration) of releases are also a critical factor. Depending on the discharge conditions, utility personnel has to implement dechlorination to prevent any negative impact. Flow/duration conditions are defined as low volume (< 50 gpm, < 2 hrs), medium volume (50-500 gpm, 2-24 hrs) or high volume (>500 gpm, >1 day).



Dechlorination implementation

Depending on the discharge conditions, utility personnel have to determine whether chemical dechlorination is required, or if chlorine will dissipate after land/pavement application and travelling in storm sewer (low flow, far from receiving stream) (See graph).

- Super-chlorinated waters are typically dechlorinated using a solution of sodium thiosulfate. When using tablets, water must be discharged at a very low flow so that the dechlorinating chemical has time to neutralize chlorine present at high concentration. It is critical to monitor and record chlorine levels in the dechlorinated water. A DPD field kit can be used to determine the disinfection residual levels after dechlorination (pink color with chlorine).
- High volume of water with a low chlorine level requires chemical dechlorination (large water main break, tank draining, distribution system flushing, etc.).
- Medium volumes of water with low chlorine residual do not require chemical dechlorination unless there is a receiving stream / pond nearby the chlorinated water discharge. When discharging water within 500 yards of a receiving stream, discharged water should be dechlorinated.
- For low volume water with low chlorine residual, chlorine dissipates after land/pavement application, and in the sewer lines. No chemical dechlorination should be needed.
- When In doubt, dechlorination should be implemented.

Chemical dechlorination with Dechlor mat/strip.

This method can be used for dechlorination of discharges from trenches during main breaks or during flushing. The tablets (sulfite tablets or ascorbic acid tablets) are placed inside synthetic mesh fabric pockets sewn together in a grid or line. The dechlor mat is laid across the flow path or over the storm drain and either weighed down, or securely attached with a rope, to keep it in place. As the discharged water flows over and around the tablets, chemical is released and neutralizes chlorine.

Procedure:

- Fill pockets of dechlor mat/strip with dechlor tablets. If the pocket contains a partially used tablet, add another tablet only if there is room.
- Place the dechlor mat in the flow path and secure with weight or rope.
- Periodically check to ensure that some tablet remains in each pocket, and that the water flow is crossing the mat. Chlorine residual should also be monitored (DPD test) and recorded to ensure there is no chlorine residual after water has crossed the mat (no pink color). If there is pink color, add additional mat/strips or decrease flow (if possible).
- Clean up. When discharge is complete, remove tablets and hose the Dechlor mat/strip. Store clean Dechlor mat in a container in the field vehicle.

When flushing hydrants, a dechlorination diffuser can be connected to hydrants. Ensure the diffuser does not run out of tablets. Check and record chlorine levels after dechlorination.

Backflow and Cross Connection Program



1- Definition of backflow and cross connection

Backflow: reverse flow of water or other substances into the drinking water distribution system.





Requirements for cross connection control

Examples of various backflow preventers available



Our cross connection control program requires "containment"





3- WAYS TO PROTECT FROM BACKFLOW

Air gap



Pressure Vacuum BreakerDescription: The Pressure Vacuum Breaker (PVB)is a device consisting of one independentlyoperating spring loaded check valve.Typical use: Ideal for health hazards to protectsagainstback-siphonageundercontinuouspressuresuch asirrigationsystemsandindustrial process water systems where the waterenters the equipment.	Pressure Vacuum Breaker * Inspection Required Annually Pressure Vacuum Breaker (PVB) Normal Flow Pressure Vacuum Breaker (PVB) Normal Flow Image: Condition Installation and testing requirements: Commonly used in outdoor applications for protection in high or low hazard situations. Annual testing required
Residential dual check Description: The Residential Dual Check (RDC) provides protection by closing two internal check valves whenever the water flow stops or reverses direction. Typical use: The Residential Dual Check valve is designed for non-health hazard residential water systems.	Installation and testing requirements: A Residential Dual check is installed immediately downstream of the residential water meters, this device needs to be rebuilt or replaced ever 5 years. There is no annual testing required.
Hose connection vacuum breaker (HVB) Description: The spring-loaded check valve in the HVB does not allow drainage of water from between the hose bib and the upper part of the HVB. This device works to prevent backflow to the water supply by venting water to the atmosphere (onto the ground) when backflow conditions exist. Typical use: The most common places HVB are installed, are sillcocks (where garden hoses connect), and laundry sink faucets.	Hose Bibb Vacuum Breaker - Must Be Installed 6* Above Ground Installation and testing requirements: A hose connection vacuum breaker, or HVB are installed on your outside faucets to prevent backflow of contaminated water into the water supply. No annual testing is required.

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4a - TYPICAL FACILITIES AND RECOMMENDED BACKFLOW PREVENTERS Commercial and industrial customers

Automatic car washes	RP
Auxiliary water systems/interconnected with the public water system	RP
Exterminators/veterinary clinics	RP
Boilers/ condenser water or chilled water systems	RP
Fire systems containing chemical additives	RP
Hospitals, medical clinics, dental clinics, health clinics, sanitariums morgues mortuaries, autopsy facilities, nursing and convalescent homes	RP
Irrigation systems and lawn sprinkler systems ,if chemicals are used	RP
Laboratories /photography/medical Labs	RP
Commercial laundries	RP
Radiator and battery shops	RP
Laundries & dry cleaning	RP
Wastewater treatment plants pump stations and storm water pumping facilities	RP
Marinas and dockside facilities	RP
Commercial, swimming pools	RP
Commercial farms using pesticides and herbicides	RP
Establishments holding livestock for sale or slaughter etc	RP
Machine Tools/ Metal Processing Plants	RP
Paper Product Plant	RP
Petroleum Processing or Storage Plants	RP
Power Plant	RP
Pharmaceutical or Cosmetic Plants	RP

Buildings with (4 units or more)/ Building with water booster pumps	DC
Restaurants/ cafeterias/ suspected medium hazards	DC
Fire sprinkler systems (without chemicals)	DC
Multi-use commercial, office or warehouse facilities	DC
Fast-food marts and other food handling facilities with isolation valves on	DC
fountains and CO ₂ tanks	
Residential units with 2-3 units	RDC
All bookflow provention accomplian are required to be ASSE enproved BB: reduce process	

All backflow prevention assemblies are required to be ASSE approved. RP: reduce pressure zone assembly, DC: Double Check, RDC: residential dual check

Customers may request a physical inspection to determine the type of backflow preventer required When installing a RDC, it is recommended to install an expansion tank to prevent any water pressure buildup inside the premise.

Facilities having booster pumps shall be equipped with a low suction pressure cut-off device to shut off the pump when the pressure in the waterworks drops to a minimum of 10 PSI.

Fire services	Double check Detector Backflow Double check Detector Backflow (DCDA) Preventers are designed to protect against both back-siphonage and backpressure. The DCDA incorporates a meter by-pass to detect leaks and unauthorized water usage.
Irrigation system	Reduced pressure Zone (RPZ) assemblies, Pressure Vacuum Breaker (PVB)



AMERICAN WATER 4b- EXAMPLES OF POTENTIAL CROSS CONNECTION SITES IN COMMON FACILITIES

Facility	Potential Cross connection sites
Restaurants	Beverage systems using CO ₂ tanks
	Dishwashers
	Steam cooking systems
	Preparation areas
Health and Fitness	Swimming pools/ Saunas/ Jacuzzi's / whiripools
Center	Boller systems
Parks / Aquariums /	Viewing tanks
Zoos	Pools/ Ponds
2000	Animal feeding systems
Schools / Colleges	Laboratories- chemistry, physics, biology
	Swimming pool
Hospital / Dental	Aspirators
Offices / Morgue/	Central suction units
Funeral Home /	Spit sinks
Veterinarians	Laboratories
	Autoclaves
	Pipette washer
	Photo Labs – Xray developing

5- RESIDENTIAL CUSTOMER REQUIREMENTS



The State requires each public water system owner to establish a program for cross connection control and backflow prevention. Such program is designed to prevent contamination of drinking water.

Contamination of the drinking water may occur under backsiphonage or backpressure conditions, whereby contaminants are siphoned or forced back into the drinking water supply. Backsiphonage of contaminants may occur when there is a pressure drop, creating a suction or partial vacuum in the system. It may happen during a waterline break or high usage in the water system, such as heavy consumption of fire fighting situations (when fire hydrants are available). Backpressure may occur when there are pumps or boilers on the water system, which produce pressures higher than water system pressures.

At the residential level, various means of protection are available to protect against backflow and avoid contamination of the drinking water supply



Residential	facility
Potential Cross connection sites	Protection
Swimming pool	Air gap separation between water supply line and top edge of
	swimming pool
Hose bib connectors (outside water spigots)	Hose bib vacuum breaker or atmospheric vacuum breaker
where water aspirators are used to spray	downstream of the last cutoff valve.
chemicals and detergents.	
Water softeners	Air gap separation between water supply line and brine tank
Frost-proof hydrants	An approved hydrant or approved backflow preventer (pressure-
	type vacuum breaker, double gate double check valve, or other
	system recommended by manufacturer) in the waterline leading to
	the hydrant.
In-ground lawn sprinklers	Atmospheric or pressure-type vacuum breaker
Connections to other water sources such as	NO connection is allowed between the public and private water
springs, individual wells, cisternsetc	supply
Hose bibs at laundry tub	Hose bib vacuum breaker or atmospheric vacuum breaker
	downstream of last cutoff valve
Booster pump	Low pressure cut-off switch on pump suction line or other device,
	depending on installation
Storage tank (other than hot water tank)	Air gap separation between water supply outlet and top edge of
	tank
Photo developing sink	Air gap separation between water supply outlet and top edge of
	sink, or atmospheric vacuum breaker downstream of last cutoff
	valve
Lawn irrigation system	Pressure vacuum breaker (PVB), or reduced pressure principle
	assembly (RPZ)

Premises No.	Meter Number:
Please check the box that best describes your facility type: Agricultural/Farm Restaurant/Groce Food Process Correctional Facilities/ Mortuary Institutions Car Wash SingleFamily/Priv Residence Day Care/Schools/ Church/regligious Universities Universities purposes Condominium Veterinarian/Pet Shop/Aquarium Nursing homes/Senior Apartment with Centers First Response - Fire/ Multi-Family Police OPA and Government Multi Commercial Offices Golf Course Barber and Beaut Salons	 Please check the box or boxes that best describe the usage of your water in your facility: Typical, such as bathrooms, drinking fountains, outside water faucets, household laundry or dishwashing appliances Private well(s) supplying any part of your facility Connected into a manufacturing process Connected into a chemical process or photo processing Connected into a swimming pool Connected into a water operated/cooled equipment/appliances/boilers
□ Laundromat □ Medical Facilities	
□ Other	
 Please check the box or boxes that best describe y protection at your facility: This account serves private hydrants only (no fire systems in facility) This account serves an installed fire sprinkler system Fire sprinkler system has outside fire department con for pumping into system 	Your fire Existing backflow device information is needed if you have a backflow device installed on your plumbing. Please complete below and provide a copy of the most recent backflow test report(s). (If no backflow device is installed on your plumbing please skip this section.) Please circle type: RDC, RP, RPDA, DC, DCDA, PVB, SVB, AVB Manufacturer:
Eiro sprinkler system contains antifraaza er other shar	
□ None	Please circle type: RDC, RP, RPDA, DC, DCDA, PVB, SVB, AVB Manufacturer: Serial #: Model: Online to: Location
If backflow prevention devices are installed on your plumb sprinkler system, it is required that they be tested annua copies of the test reports be maintained on file with the company. If you do not have current copies of the test repo- file with us, please attach to this survey.	ing/fire Please circle type: RDC, RP, RPDA, DC, DCDA, PVB, SVB, AVB Ily and Manufacturer: water Serial #: orts on Model: Online to: Size: Location
Signature of individual completing this survey:	Phone#
Email(optional):	Date

Virginia American Water Cross Connection Department 2223 Duke Street Alexandria, VA 22314 Phone: 703-447-3133 Email: VA.MD.CCC@amwater.com

SECTION 01300

SUBMITTALS

PART 1: GENERAL

1.01 GENERAL

- A. Within 10 days after the Effective Date of the Agreement, unless otherwise specified, Contractor shall submit to Engineer for timely review:
 - 1. All submittals defined in the Contract Documents or as requested by the Engineer including Shop Drawings, certifications and manufacturers information requiring approval by the Water Company.

1.02 CONSTRUCTION SCHEDULE

- A. Contractor shall submit weekly, a fourteen-day look-ahead schedule that identifies planned work activities and crew locations within the District.
- B. In addition to the weekly progress schedule, prepare and submit detailed progress schedules for the entire project, schedule of values and shop drawing and sample submittal schedules to the Engineer for approval in accordance with Paragraphs 2.6 and 2.9 of the General Conditions. The schedule shall be in bar graph form and shall include, as a minimum, the following separate activities:
 - 1. Physical construction (identifying mobilization, demobilization, setup time, lags, etc.).
 - 2. Issuance by Contractor of purchase orders for material and equipment and submittal of shop drawings and samples to the Engineer.
 - 3. Review by Engineer for each submittal of samples and shop drawings. Unless otherwise approved by the Engineer, allow ten (10) working days for Engineer to review each submittal.
 - 4. Fabrication time for materials and equipment.
 - 5. Delivery of materials and equipment.
 - 6. Installation of materials and equipment.
 - 7. Testing, start-up, and training for individual pieces of equipment or entire systems as appropriate.
 - 8. Weather affected activities.
 - 9. Outages or interruptions of Owner's facilities required to perform work.
 - 10. Demolition or removal work under this Contract.
- C. Activity durations shall represent the best estimate of elapsed time considering the scope of the Work involved in the activity and the resources planned for accomplishing the activity expressed in working days.
- D. Activity descriptions shall clearly define the scope of work associated with each activity.

- E. Detail the construction work schedule to an extent that progress can be readily monitored on a weekly basis. In general, the construction work shall be detailed such that no construction activity shall have duration greater than fifteen (15) workdays. As a minimum, each activity shall be coded by:
 - 1. Activity type (i.e., submittal, Engineer's review, material order material delivery, pilot hole drilling, well testing, development, etc.).
 - 2. Responsibility (i.e., Contractor, subcontractor A, subcontractor B, Owner, Engineer, etc.).
 - 3. Area (i.e., Pilot Wells, Production Wells, sitework, etc.).
- F. Develop the construction schedule as necessary to properly control and manage the project. The above schedule development requirements are a minimum.
- G. The preliminary progress schedule shall be submitted in a bar graph format and shall include, as a minimum, a graphic representation of all significant activities and events involved in the construction of the project. The graphic representation and statement must clearly depict and describe the sequence of activities planned by the Contractor, their interdependence and the times estimated to perform each activity.

1.03 FINALIZING SCHEDULES

- A. Prepare to present and discuss at the preconstruction meeting, the schedules submitted in accordance with this specification. Unless additional information is required to be submitted by the Contractor, the Engineer will, within 15 working days of the preconstruction conference, provide comments to the Contractor. Then resubmit the affected schedules addressing the Engineer's comments.
- B. Approval of the final schedules by the Engineer is advisory only and shall not relieve the Contractor of responsibility for accomplishing the work within the Contract Times. Omissions and errors in the approved schedule shall not excuse performance less than that required by the Contract. Approval by the Engineer in no way makes the Engineer an insurer of the success of those schedules or liable for time or cost overruns flowing from shortcomings in such schedules.

1.04 REQUIREMENTS FOR CONFORMING TO SCHEDULE

A. Take such steps as will be necessary to improve progress, if, in the opinion of the Engineer, the Contractor falls behind the progress schedule. Engineer may require Contractor to increase the number of shifts and/or overtime operations, days of work, and/or the amount of construction planned, and to submit for approval such supplementary schedule or schedules as may be deemed necessary to demonstrate the manner in which the agreed rate of progress will be regained, all without additional cost to the Owner. An updated cash flow schedule will be required in this occurrence and will be provided with the supplementary schedules referenced above.

1.05 UPDATING SCHEDULES

- A. Submit to the Engineer monthly updates of the schedules required per this specification section. Be prepared to discuss the monthly update and the subsequent monthly job meeting if such meetings are to be held.
- B. Progress and shop drawing schedule updates shall reflect the progress to date by providing actual start dates for activities started, actual finish dates for completed activities, and identifying out of sequence work, schedule logic changes and any circumstances or events impacting the current schedule. The updates shall also contain the Contractor's best estimate of the remaining duration for activities not complete as of the date of the update. All graphic presentations and other information required per the initial submittal of these schedules shall be provided with each update.

1.06 ADJUSTMENT OF PROGRESS SCHEDULE AND CONTRACT TIMES

- A. If the Contractor desires to make changes in the method of operating which affect the approved progress schedule, notify the Engineer in writing stating what changes are proposed and the reason for the change. If the Engineer approves these changes, revise and submit for approval, without additional cost to the Owner, all of the affected portions of the schedule.
- B. Shop drawings and samples which are not approved on the first submittal or within the schedule time shall be immediately rescheduled, as well as any work which fails to pass specified tests or has been rejected.
- C. The Contract Times will be adjusted only for causes specified in the General Conditions. In the event the Contractor requests an adjustment of the Contract times, furnish such justification and supporting evidence as the Engineer may deem necessary for a determination as to whether the Contractor is entitled to an adjustment of Contract Times under the provisions of the General Conditions. The Engineer will, after receipt of such justification and supporting evidence, make findings of fact and will advise the Contractor in writing. If the Engineer finds that the Contractor is entitled to any adjustment of the Contract Times, the Engineer's determination as to the total number of days adjustment shall be based upon the currently approved progress schedule and on all data relevant to the adjustment. The Contractor acknowledges and agrees that actual delays in activities which, according to the progress schedule, do not affect the Contract completion date shown by the critical path in the schedule will not be the basis for an adjustment of Contract Times.
- D. From time to time it may be necessary for the progress schedule and/or Contract Times to be adjusted by the Owner to reflect the effects of job conditions, weather, technical difficulties, strikes, unavoidable delays on the part of the Owner, and other unforeseeable conditions which may indicate schedule and/or Contract Times adjustments. Under such conditions, the Engineer shall direct the Contractor to reschedule the work and/or Contract Time to reflect the changed conditions. Revise the construction schedule accordingly. No additional compensation shall be made to the Contractor for such changes except as provided in the General Conditions. Unless otherwise directed, take all possible

actions to minimize any extension to the Contract Times and any additional cost to the Owner.

1.07 SHOP DRAWINGS

- A. Promptly supply to the Engineer for approval, shop drawings with details and schedules for all items as noted in the Drawings and/or Specifications and/or required by the Engineer. Submittals are required for all equipment and materials to be installed on the job.
- B. Electronic copies of all drawings, schedules and brochures shall be submitted for approval. Black line prints or blue line prints are required. Blueprints (white lines on a blue background) are not acceptable. Each submittal shall have the job name on it.
- C. Submittals smaller than $8\frac{1}{2}$ by 11 inches shall be secured to paper $8\frac{1}{2}$ by 11 inches.

1.08 PRE-CONSTRUCTION VIDEO/ELECTRONIC PHOTOS

- A. Prior to mobilization at the site, furnish to the Engineer on DVD a video recording of all planned construction areas, material storage areas, areas adjacent to these areas, including but not limited to, streets, driveways, sidewalks, curbs, ditches, fencing, railing, visible utilities, retaining structures and adjacent building structures. The purpose of the video is to document existing conditions and to provide a fair measure of required restoration. Care should be taken to record all existing conditions which exhibit deterioration, imperfections, structural failures or situations that would be considered substandard. Notify the Engineer when the video is to be taken to provide the Engineer an option to be on site during the documenting of the project area.
- B. The video shall be high quality, color and in an approved electronic format. Temporary lighting shall be provided as necessary to properly video areas where natural lighting is insufficient (indoors, shadows, etc.). The video shall include an audio soundtrack to provide the following information:
 - 1. Detailed description of location being viewed referenced to Contract Drawings (i.e., well location, building designation, pipeline route, etc.)
 - 2. Direction (N, S, E, W, looking up, looking down, etc.) of camera view
 - 3. Date, time, temperature, environmental conditions during recording.

Where required by Engineer, electronic photographs of specific locations shall be provided to supplement the electronic video.

C. Any areas not readily visible by video/photo methods shall be described in detail. Unless otherwise approved by Engineer, video shall not be performed during inclement weather or when the ground is covered partially or totally with snow, ice, leaves, etc. D. As many recordings or photos as are necessary to satisfy the requirements of this section shall be prepared. The original documents shall be submitted to the Engineer accompanied by a detailed log of the contents of each DVD. The log should include location descriptions with corresponding file name to facilitate the quick location of information contained on the DVDs. The DVDs will be maintained by the Engineer during construction and may be viewed at any time by Contractor upon request. Upon final acceptance, the DVDs will become the permanent property of the Owner.

1.09 PROGRESS AND FINAL PAYMENTS

A. After Contractor has, in the opinion of Engineer, satisfactorily completed all corrections identified during the final inspection and has delivered, in accordance with the Contract Documents, all maintenance and operating instructions, schedules, guarantees, bonds, certificates or other evidence of insurance, certificates of inspection, marked-up record documents (as provided in Paragraph 6.12), and other documents, Contractor shall make application for final project close out..

1.10 CONTRACTOR'S DAILY REPORTS (If required by Engineer)

- A. Prepare and submit daily reports containing the following information:
 - 1. The number of craftsmen and hours worked of each subcontractor,
 - 2. The number of hours worked by each trade,
 - 3. The number of hours worked of each type of equipment,
 - 4. A description of work activities performed,
 - 5. A description of any material or equipment deliveries,
 - 6. Description of obstructions encountered,
 - 7. The temperature and weather conditions,
 - 8. Downtime due to equipment failure,
 - 9. Detail cause for work delays,
 - 10. If applicable, for excavation beyond the dimensions of the standard pipe trench (greater than 5 feet in depth), submit photo documentation if an Owner inspector is not on site to verify the depth of excavation.
- B. The daily reports shall be submitted to the Engineer or Resident Project Representative on a daily basis, by the end of the next business day.
- C. Information provided on the daily report <u>shall not</u> constitute notice of delay or any other notice required by the Contract Documents. Notice shall be as required therein.

1.11 AS-BUILTS

Where identified as a product of the work, provide as-built drawings adhering to the criteria provided here and that found in the special conditions.

- A. Templates All measurements and information shall be recorded on templates provided. No other backgrounds, templates nor formats will be accepted for the As-Built submission.
- B. Recording the Information Provide the Record As-Built and Field Sketch information in PDF format.
- C. Coordinates Provide the required survey coordinates in the State Plane Coordinate System unless otherwise noted. The drawing features included shall be as noted below (See 'Pipeline As-Built Drawing Procedure').
- D. Submitting the Information When the Record information is ready, submit digital copies of all the information, including sketches to the Engineer for approval. The electronic information shall be emailed as a zip file.
- E. The Information Process The Engineer will approve the submission or 'red line' any information needing to be corrected or added, and return it for resubmission. When the submittal is approved by the Engineer, provide a zip file containing approved As-Built drawings and field cards in PDF format.

Initial submission must be provided within (14) calendar days of the 'Construction Completion' date, not including the restoration work. The Engineer will return the submission within (7) calendar days of receipt. The approved final submission must be provided within twenty-eight (28) calendar days from the 'Construction Completion' date, not including the restoration work.

- F. General information required At a minimum, all As-Built record drawings shall contain the following information:
 - 1. North Arrow
 - 2. Face of curb lines, easement lines, edge of pavement (EOP) or right-ofway lines
 - 3. Plate Map number
 - 4. Any known conflicting utilities
 - 5. All objects located shall be referenced to other objects with (2) perpendicular measurements. All such measurements shall be from permanent existing structures, such as catch basins, manholes, buildings, etc. (no utility poles)
 - 6. The proposed pipeline 'line' designation shall be shown in bold or heavier line style per template and sample.
- G. Title Sheet At a minimum, all As-Built recording drawings shall contain the following formation:
 - 1. American Water District and Project Name
 - 2. Project Business Unit Number
 - 3. Design Consultant Engineering Company Name
 - 4. Project Date
 - 5. County and Town
 - 6. List of Drawings
 - 7. Drawing Key with corresponding drawing reference

- 8. WBS Element (information provided by Engineer)
- 9. Name of Contractor and Construction Inspector (information provided by Engineer)
- H. Water Features At a minimum, all As-Built recording drawings shall contain the following formation:
 - 1. Each new valve, hydrant, fitting, and, if applicable, corporation stop and meter pit, shall be annotated
- I. Water Pipeline information required At a minimum, all As-Built record drawings shall contain the following information:
 - 1. Title Block Information completed
 - 2. Each drawing shall include only the work along one street block (transmission mains excluded) and include the intersecting street corners with the distance to the center line of each intersection. Include Match Lines if multiple drawings are required.
 - 3. If more than one drawing is required, include an overall site plan of the whole project with a drawing key
 - 4. Pipe diameter and material
 - 5. Bill of Materials
 - 6. Date the water main was put 'In-service' (data provided by Engineer)
 - 7. Reference the Point of Connection where the new main pipeline connects to existing Owner facilities and provide dimensions to nearest existing appurtenance
 - 8. If project continues from an existing stub, a dimension from the center line of the nearest street intersection and existing line valve shall be included. Provide coordinates for the referenced existing valve.
 - 9. If the project is a continuation of a previous project, reference the previous project reference number
 - 10. All valves, tees, horizontal/vertical bends, and the start and end of the new water main shall be dimensionally located
 - 11. All connections, wet cuts, and fittings not required to have coordinates shall be dimensionally located
 - 12. Indicate abandoned pipe with type of material and length (if applicable)
 - 13. Indicate and locate buried valves (if applicable)
 - 14. Provide measurement from face of curb or edge of pavement at every 250 feet maximum along the pipeline
 - 15. At abrupt changes in pipe elevation, provide a referenced drawing showing the profile of the work and list the material used
 - 16. Provide the depth from finish grade to top of pipe every 100 linear feet, and at the start and end of the new water main
 - 17. Name of Contractor and Construction Inspector (full last name) on the project (locate in title block)
- J. Water Transmission Pipeline Information Transmission Mains are typically 16" in diameter and larger; however, the Engineer may classify some 12" diameter pipe projects as a transmission main. Transmission main as-built drawings shall include all relevant information noted above and the following:

- 1. Title Sheet (see Section 1.11.G above)
- 2. Include both Pipeline plan and profile views on the same sheet. Provide a detail sheet copying all valve cards (data provided by Engineer) listed those included and not included on the plan/profile sheets
- 3. Include drawing details of all interconnections
- 4. Provide the Manufacturer data for the pipe, fittings, and appurtenances on the drawings
- 5. Show and identify all restraint locations
- 6. Valves, bends, tees, and top of main elevation every 300 feet maximum shall be dimensionally located
- K. Water Connection (Tap and Service) Drawing Information Service drawings are required where services currently do not exist. This drawing can be incorporated into the Pipeline Drawing noted above. Service drawings shall be on the 11 by 17 inch template. The drawing shall contain the general information above and the following additional information:
 - 1. Title Block information completed
 - 2. Every service connection, service valve or curb stop, if installed, shall be located dimensionally with separate measurements for both the corporation and curb/meter box
 - 3. Valves shall be dimensionally located
 - 4. Identify the main pipeline size, type and location from nearest face of curb or edge of pavement
 - 5. Tap number and house address shall be clearly shown at each location
 - 6. Show the size, length and service material
 - 7. Match lines and/or drawing key if more than one sheet
- L. Water Field Sketches Some items installed required separate detailed field sketches. This includes the following:
 - Valves (including Valves for Blow-offs) (Valve cards) Valve location measurements and information shall be shown on an 8½ by 11 inch sketch using the template provided. Separate sketches are required for each valve, regardless of their proximity to each other. The sketch should be an enlarged and more detailed version of what is depicted on the Pipeline drawing. Any 'Blow-offs' installed with the work shall be shown in detail on a Valve sketch with the same level of information as a valve. At a minimum, all Valve sketches shall contain the following:
 - a. Manufacturer, type, open direction, number of turns, normal position, and current position (confirm open direction upon delivery)
 - b. Main Pipeline type and size
 - c. Valve Category (Distribution Automatic, Distribution Manual, Transmission Automatic, Transmission Manual, Hydrant Aux, Service Automatic, Service Manual, Blow-Off Automatic, Blow-Off Manual)
 - d. Valve Type (Altitude, Angle, Ball, Butterfly, Check, Double Check, Gate, Globe, Plug, Pres Relief, Pres Reg, RPZ, Solenoid,

Tapping, Telescopic, Level Control, Combination Air, Curb Stop, Pres Vacuum Breaker, Vacuum Relief, Air Release)

- e. Valve Application (Air Release, Altitude, Back Flow Prevention, Blow-off, Booster Discharge, Bypass, Chemical, Crossover, Distribution in Grid, Distribution Dead End, Drain, Fire Service, Hydrant Aux, In Plant, Isolation, Plant, Plant Discharge, Pump Discharge, Pump Suction, Raw Water, Service Line, Transmission, Vacuum Breaker, Zone Separation)
- f. Access Type (No Entry, Valve Box, Manhole, Vault, Stop Box, Pit Box, Building)
- g. Water Type (Potable Water, Raw Water, Fire Only, Grey Water, Dry, Non-Potable, Sample, Residuals)
- h. Date the Valve was put 'In-service'
- i. Valve identifying number (data provided by Engineer)
- j. Identify other valves, hydrants, fittings and blow-offs within the immediate vicinity
- k. Identify permanent existing structures
- I. At least (2) tie down measurements to valve from permanent existing structures including catch basins, manholes, buildings, curbs, etc. (no utility poles)
- 2. Hydrant Submit hydrant location measurements and information on an 8½ by 11-inch sketch. Each 'hydrant' shall have a separate sketch. The sketch should be an enlarged and more detailed version of what is depicted on the Pipeline drawing. At a minimum, all Hydrant sketches shall contain the following:
 - a. Manufacturer and hydrant number (data provided by Engineer)
 - b. Bill of Material
 - c. Record flow test results on sketch. If no test was required, record static pressure (data provided by Engineer)
 - d. Hydrant Type (*Fire, Pit, Flushing*)
 - e. Hydrant Size
 - f. Hydrant Valve ID (provided by Engineer)
 - g. Main Pipeline and lateral type, size, and material
 - h. Date the Hydrant was put 'In-service'
 - i. Identify other valves, hydrants, fittings and blow-offs within the vicinity
 - j. Identify permanent existing structures
 - k. If an existing hydrant was relocated, reference the old hydrant number and its BU (data provided by Engineer)
- 3. Tap (Service Connections Installed) Tap location measurements and information shall be shown on an 8½ by 11-inch sketch. Each 'Service' shall have a separate Tap sketch. The sketch should be an enlarged and more detailed version of what is depicted on the Pipeline drawing / Service drawing. At a minimum, all Tap sketches shall contain the following:
 - a. Locate dimensionally the identified Service/Tap

- b. Sketch shall be oriented with the building receiving the service at the top of the sketch.
- c. Locate dimensionally the tapped water main from nearest face of curb or EOP
- d. Locate dimensionally the curb/meter box from nearest curb or EOP
- e. Tap identifying number (data provided by Engineer)
- f. House address number and Lot & Block number when applicable (data provided by Engineer)
- g. Length of 'Service'
- h. Valve ID Number (data provided by Engineer)
- i. Service to Service dimensions if less than 100 feet
- j. Identify anything that is underground within (6) feet of the service tap (i.e. blow-offs, chlorine tap, electric, gas, etc.)
- k. Separate measurements for both the corporation and curb/meter box
- I. At least (2) tie down measurements to curb/meter box from permanent existing structures including catch basins, manholes, buildings, curbs, etc. (no utility poles)
- m. When a service is renewed, the sketch should be labeled "Renew and Increase" and the customer's size and type of material should be recorded
- n. Bill of Material used
- o. Depth of service at curb

PART 2: PRODUCTS

2.01 TESTING DATA CERTIFICATES

A. Product testing shall comply with all respective AWWA standards. The certificates of compliance shall be electronically scanned and submitted by E-mail to the Engineer or by submitting the hard copy originals to the Engineer.

PART 3: EXECUTION

Not Used.

END OF SECTION

Section 01300 Submittal: As-Built Requirements



Project Name:	
District:	
In-Service Date:	

Please check all boxes below that have been completed and have all unchecked boxes corrected.

Valve Cards

- □ Are valve cards (using VAAW template) for each new valve installed included?
- Does the valve have the correct ID?
- □ Is the existing infrastructure included in the sketch?
- Does the card include two measurements from permanent existing structures to the valve?

Are the following main attributes present:

- Date in Service
- Open Direction
- □ Number of Turns
- □ Nut Type
- □ Access Type
- Current and Normal Position
- □ Make and Size
- □ Valve Type, Application and Category

Service Cards

- □ Are service cards (using VAAW template) for each new service installed included?
- □ Is the existing infrastructure included in the sketch?
- □ Is the service tap, meter and/or curbstop included in the sketch?
- □ Located dimensionally the tapped water main from nearest face of curb or EOP?
- Located dimensionally the curb/meter box from nearest curb or EOP?
- □ Is the address included?
- □ Is the length of service included?
- □ Are all assets (blow-offs, electric, gas, etc.) within 6 feet of the service tap identified in the sketch?
- Does the card include two measurements from permanent existing structures to the meter pit and/or curbstop?

Are the following main attributes present:

- Type: Domestic, Hydrant Lateral, Fire, Irrigation, Auxiliary or Combined
- Material and Diameter
- Depth at curb
- Meter Size

As-Built Plan Set

- Was the as-built created using the VAAW as-built sketch template?
- Are individual sketches for each street block included?
- Are all features attributed with their correct ID?
- □ Is the bill of materials included?
- Are abandoned pipes with material and size included?
- Are buried valves included in the sketch?
- □ Are all new valves, hydrants, mains, service lines, fittings, meters, corporation stops included in the sketch?
- □ Is the in-service date included?
- □ Is the trench information included?

Are the following main attributes present:

- Material and Diameter
- □ Joint Type
- Length between fitting(s), valve(s), start/end of encasement(s)/restraint(s), material change(s), and/or any other notablefeature(s)

Hydrant Cards

- Did you fill a hydrant card (using VAAW template) for each new hydrant installed?
- Does the hydrant have the correct ID?
- □ Is the existing infrastructure included in the sketch?
- Does the card include two measurements from permanent existing structures to the hydrant?

Are the following main attributes present:

- Date in Service
- □ Hydrant Type
- Hydrant Barrel Diameter]
- Hydrant Valve ID
- □ Manufacturer
- Water Main and Lateral Size and Material
- Lateral Type

VALVE INFORMATION

INSTALLATION INSPECTION

*
VIRGINIA
American Water

Date In Service			Valve Number:
Valve Opens	Right	Left	SAP Equipment Number:
No. Turns			District
Nut	Square	Tee Wheel	Town
Access Type			Street
Current Position	Open	Close	Cross Street
Normal Position	Open	Close	Inspected By
Make			Task/WBS No.
Valve Size			Location Information
Valve Type			
Valve Application			
Valve Category			

SKETCH

SERVICE INFORMATION

INSTALLATION INSPECTION



Date In Service	Premise Number:	
Service Type	District	
Service Size	Town	
Service Material	House Number	
Length of Service	Street	
Depth at Curb	Cross Street	
Valve Number	Inspected By	
Meter/Curb Stop Size	Task/WBS No.	
NORTH ARROW	Location Information	

SKETCH

	PROJECT NAME		STREET	В	LOCK	WBS/WO	VAAW DISTRIC
	DATE STARTED	DATE COMPLETED		DATE FLUSH	ED	DATE IN SERVICE	
AS-BUILT	TRENCH: AVER. DEPTH	AVER. WIDTH		SOIL TYPE	ROCK TY	PE PAVING: KIN	D
	OVERALL LENGTH OF MAINS INSTALLED		SIZE	MATERIAL			
SKETCH	OVERALL LENGTH OF MAINS IN	STALLED	SIZE		MATERIAL		

СТ

LENGTH CUT

WIDTH CUT

QTY		MATERIAL			
DRAWN BY:					
APPROVED BY:					
INSPECTOR					
Sheet		Of			

HYDRANT INFORMATION



Date In Service			Hydrant Number:		
Hydrant Type			SAP Equipment Number:		
Hydrant Size			District		
Manufacturer			Town		
Hydrant Valve ID			Street		
Water Main Size			Cross Street		
Water Main Material			Inspected By		
Lateral Size			Task/WBS No.		
Lateral Material			Location	Information	NORTH ARROW
Lateral Type					
Hydrant Barrel Diameter					
Open Direction	Right	Left			

SKETCH

SECTION 01600

PRODUCTS

PART 1: GENERAL

1.01 PROTECTION OF MATERIAL AND EQUIPMENT

- A. Provide for the safe storage of all material furnished or purchased until it has been incorporated in the completed project and accepted by the Engineer. Bear the risk of loss and/or damage to the materials and Work until the Work is finally accepted by the Engineer.
- B. All electrical and mechanical equipment shall be stored in a warm, dry shelter with proper ventilation. Under no circumstances shall motors, electrical control equipment or any other electrical or mechanical equipment be stored under polyethylene plastic covers or tarpaulins. When space is available inside existing structures, and the Owner approves, the Contractor will be allowed to store equipment inside them. Should such space not be available, construct a shelter with a source of heat and proper ventilation as approved by the Engineer for the storage of equipment.
- C. The interior of all pipe, fittings, and accessories shall be kept free from dirt, foreign matter and standing water at all times.
- D. After valves and hydrants have been inspected, properly store them prior to use. In order to prevent entry of foreign material that could cause damage to the seating surfaces, the valves and hydrants shall be stored in a fully closed position unless recommended otherwise by the manufacturer. Resilient seated valves shall be stored in accordance with the manufacturer's recommendations. This may include storage with protective covers for rubber seats and in marginally open condition. Valves and hydrants shall be stored indoors unless otherwise approved by the Engineer.
- E. If valves must be stored outdoors, protect the operating mechanism, such as gears, motor, actuators and cylinders, from weather elements. Valve ports and flanges must be protected from the weather and foreign materials. If valves are subject to extreme (freezing or excessively hot) temperatures, all water must be removed from the valve interior and the valve closed tightly before storage, unless specifically recommended otherwise by the manufacturer. Valves shall be stored on pallets with the discs in a vertical position to prevent rainwater from accumulating on top of the disc, seeping into the valve body cavity and freezing and cracking the casting.

1.02 SERVICING EQUIPMENT

A. Check all equipment upon acceptance to determine if oil reservoirs are full and areas to be greased are properly packed with grease. Provide the proper grease or oil for use in lubricating the required areas in the equipment. Any service to equipment while in storage, or installed pending acceptance, is the responsibility of the Contractor and shall be performed per manufacturer's requirements, industry standards or as stated specifically in the technical specifications.

1.03 RESPONSIBILITY FOR MATERIAL AND EQUIPMENT

- A. Under no circumstances shall pipe, valves, fittings, or appurtenances be dropped or dumped from any trucks or equipment. When received from the Carrier and at time of unloading, inspect all pipe and accessories for loss or damage. No shipment of material shall be accepted by the Contractor unless loss or damage has been described on the Bill of Lading by the Carrier's agent. Any discrepancies between the Bill of Lading and the physical material shall be noted on the Bill of Lading. All demurrage charges on carloads or truckloads of pipe or other material shall be paid by the Contractor.
- B. After acceptance of material and/or equipment by Contractor at point of delivery, assume full responsibility for safe and secure storage, handling, servicing and installation of such material and/or equipment in accordance with manufacturer's recommendations, industry standards or specific requirements of the Contract Documents. Once in Contractor's possession, assume full responsibility for, and protect all material from theft and damage. Any lost or stolen materials shall be replaced at the Contractor's expense.
- C. Re-inspect all material for defects, correct size, and quantity in the field prior to installation. Immediately report all material found to be defective, improperly sized, or deficient in quantity to the Owner.
- D. The Contractor is responsible for all material furnished by the Contractor and Contractor suppliers. All such material which is defective in manufacture or has been damaged in transit or has been damaged after delivery shall be replaced by the Contractor at the Contractor's expense.
- E. Certain material and equipment will be furnished by the Owner as noted in the Contract Documents. The Contractor's responsibility for material and/or equipment furnished by the Owner shall begin upon the Contractor's acceptance of such material and/or equipment at the point of delivery. All material and equipment shall be examined, and items found to be defective in manufacture and/or otherwise damaged shall be rejected by the Contractor at the time and place of delivery. The Owner will thereupon repair or replace the damaged items. Any material and/or equipment found to be defective prior to acceptance by the Engineer shall be repaired or replaced by Contractor at no additional cost to Owner unless Contractor submits proof that such defect was latent and could not have been detected by Contractor when performing their duties and responsibilities under these Contract Documents.

F. Contractor's and Owner's responsibilities for providing guarantees or warranty and manufacturer's representatives for service, inspection, certification of installation, installation, field training, start-up, etc. for material and/or equipment furnished by Owner shall be as follows unless otherwise specified: Owner will provide the warranty and Contractor is responsible for providing manufacturer's representatives for all necessary field service, start-up service, installation certifications, installation, field training of Owner's personnel, etc. for Owner furnished material and/or equipment as required for acceptance of such material and/or equipment in the completed project.

PART 2: PRODUCTS

2.01 GENERAL

Unless otherwise specifically provided for in these Specifications, all equipment, materials and articles incorporated in the work shall be new, in current production and the best grade obtainable consistent with general construction usage.

2.02 COORDINATION OF DIMENSIONS

Verify and make necessary corrections to construction dimensions so that all specified and/or alternative equipment, which is approved by the Engineer, can be installed and will function within the intent of the Contract Drawings and Specifications. Promptly notify the Engineer of all necessary corrections required.

2.03 SAFETY AND HEALTH REQUIREMENTS

- A. All materials, equipment, fixtures and devices furnished shall comply with applicable Laws and Regulations.
- B. All material and equipment furnished and installed under this Contract shall be equipped with suitable and approved safety guards and devices required for the safety of the public and operating personnel. Such guards and safety devices shall be in accord with the latest requirements of safety codes approved by the American National Standards Institute as well as the safety requirements of applicable Laws and Regulations. Where said safety codes of the ANSI are incompatible with applicable Laws and Regulations, said Laws and Regulations shall prevail.

PART 3: EXECUTION

3.01 INSTALLATION

A. Material and equipment shall be installed in accordance with the appropriate Sections of these Specifications.

3.02 SERVICES OF MANUFACTURER'S REPRESENTATIVE

A. Arrange for a qualified service representative from each company, manufacturing or supplying certain equipment as required by the individual Specification Sections to perform the duties herein described.

- B. After installation of the applicable equipment has been completed and the equipment is presumably ready for operation, but before it is operated by others, the representative shall inspect, operate, test, and adjust the equipment. The inspection shall include, but shall not be limited to, the following points as applicable:
 - 1. soundness (without cracked or otherwise damaged parts)
 - 2. completeness in all details, as specified
 - 3. correctness of setting, alignment, and relative arrangement of various parts
 - 4. adequacy and correctness of packing, sealing and lubricants
- C. The operation, testing, and adjustment shall be as required to prove that the equipment is left in proper condition for satisfactory operation under the conditions specified.

END OF SECTION

SECTION 01700

PROJECT CLOSEOUT

PART 1: GENERAL

1.01 TESTING OF FACILITIES

A. Produce a first-class job and all Work shall be tested under operating conditions and pressures. Any leaks or malfunctions shall be repaired to the satisfaction of the Engineer at no additional expense to the Owner. This provision with reference to leakage shall also apply to water tightness of buildings.

1.02 CLOSEOUT PROCEDURES

A. Submit written certification that Contract Documents have been reviewed, Work has been inspected, and that Work is complete in accordance with Contract Documents and ready for Engineer's inspection. Provide submittals to Engineer that are required by governing or other authorities.

1.03 PROJECT RECORD DOCUMENTS

- A. Maintain on site, one set of the following record documents; record actual revisions to the Work immediately upon completion of each street of pipeline installation:
 - 1. Contract drawings
 - 2. Specifications
 - 3. Addenda
 - 4. Change orders and other modifications to the Contract
- B. Store record documents separate from documents used for construction. Record information concurrent with construction progress.
- C. Specifications: Legibly mark and record at each product section description of actual products installed, including the following:
 - 1. Manufacturer's name and product model and number
 - 2. Product substitutions or alternates utilized
 - 3. Changes made by addenda and modifications
- D. Record Documents and Shop Drawings: Legibly mark each item to record actual construction including:
 - 1. Measured depths of foundations in relation to finish floor datum.
 - 2. Measured horizontal and vertical locations of underground utilities and appurtenances, referenced to permanent surface improvements.
 - 3. Measured locations of internal utilities and appurtenances concealed in construction, referenced to visible and accessible features of the Work.
 - 4. Field changes of dimension and detail.
 - 5. Details not on original Contract Drawings.

E. Submit documents to Engineer with final Application for Payment.

1.05 SPARE PARTS AND MAINTENANCE MATERIALS

A. Provide products, spare parts, maintenance and extra materials in quantities specified in individual specification sections.

1.06 GUARANTEES AND WARRANTIES

- A. The Contractor expressly warrants that all workmanship and materials performed or furnished under this Contract will conform to the Specifications, Drawings, samples and other applicable descriptions furnished or adopted by the Contractor and with all applicable laws, provisions and requirements of the Contract Documents. The Contractor shall remedy any defects due to faulty materials or workmanship which shall appear within warranty period from the later date of final restoration or acceptance of the work hereunder and pay for any damage to other work resulting therefrom. The Owner shall give notice of observed defects with reasonable promptness. The Contractor warranty hereunder is in addition to, and not in limitation of, any obligations found elsewhere in the Contract Documents, any special guarantees provided by the Contractor or his suppliers, and any obligations imposed by law.
- B. In addition to the above requirements, the Contractor shall assign material and equipment guarantees and warranties from all manufacturers and suppliers to the Owner and deliver copies of such guarantees and warranties and the assignments thereof to the Owner in order to assure the Owner of the full benefit of such guarantees and warranties.

1.07 RESTORATION

A. Restore and/or replace paving, curbing, sidewalks, gutters, shrubbery, fences, sod or other disturbed surfaces and structures to a condition equal to that before the work began and to the satisfaction of the Engineer and shall furnish all labor and materials incidental thereto.

PART 2: PRODUCTS

Not Used.

PART 3: EXECUTION

Not Used.

END OF SECTION

SECTION 02020

DEWATERING

PART 1: GENERAL

1.01 GENERAL

- A. Should water be encountered, furnish and operate pumping equipment of sufficient capacity to dewater the trench. Dewater the trench so that the laying and joining of the pipe is made in a dry environment so as to prevent water from entering the pipe during construction.
- B. No additional sum will be allowed for any reasonably anticipated dewatering operation, overtime, equipment rental or any other expense incurred due to the occurrence of ground water, surface water or water from possible leakage of existing buildings, structures and piping in the vicinity of the Contractor's operations. If Contractor believes unreasonable, unanticipated wet conditions exist, immediately contact Engineer to decide appropriate measures and to determine whether Contractor is entitled to additional compensation.
- C. Convey all trench water to a natural drainage channel or storm sewer without causing any property damage. Discharge shall be in strict accordance with state and/or local requirements.
- D. Dispose of silt and debris which accumulates during construction in strict accordance with state and/or local requirements.

1.02 PERMITS

A. The Contractor shall obtain and pay for any permits required for dewatering and disposal.

PART 2: PRODUCTS

Not Used.

PART 3: EXECUTION

Not Used.

END OF SECTION
EXISTING UTILITIES AND STRUCTURES

PART 1: GENERAL

1.01 SCOPE OF WORK

A. Certain information regarding the reputed presence, size, character, and location of existing Underground Facilities such as pipes, drains, sewers, electrical lines, telephone lines, cable TV lines, gas lines, and water lines has been shown on the Contract Drawings and/or provided in the contract documents. This information with respect to Underground Facilities is provided by the Owner in accordance with conditions described in the General Conditions and for information purposes only. Contractor is responsible to determine actual location of all utilities in proximity to the work for the purposes of the preparation of their bid and during construction.

1.02 NOTIFICATION OF UTILITIES

A. Notify the applicable State Agency with jurisdiction over underground facilities and/or all utility companies that construction work under this Contract will pass through containing their underground facilities. Notify these parties in advance to support the construction work (**minimum 72 hours**). All excavation in the vicinity of existing underground utilities shall be performed in accordance with applicable regulations.

PART 2: PRODUCTS

2.01 MATERIALS

A. Furnish all materials for temporary support, adequate protection, and maintenance of all underground and surface utility structures, supports, drains, sewer and other obstructions encountered in the progress of the work.

PART 3: EXECUTION

3.01 OBSTRUCTIONS BY OTHER UTILITY STRUCTURES

- A. Support, relocate, remove, or reconstruct existing utility structures such as conduits, ducts, pipes, branch connections to main sewers, or drains. The obstruction shall be permanently supported, relocated, removed or reconstructed where they obstruct the grade or alignment of the pipe. Contractor must do so in cooperation with the owners of such utility structures. Before proceeding, the Contractor must reach an agreement with the Engineer on the method to work around the obstruction.
- B. No deviation shall be made from the required line or depth without the consent of the Engineer.

3.02 REPAIRS

- A. Repair or replace any damage to existing structures, work, materials, or equipment incurred by Contractor's operations.
- B. Repair all damage to streets, roads, curbs, sidewalks, highways, shoulders, ditches, embankments, culverts, bridges, trees, shrubs or other public or private property caused by transporting equipment, materials or personnel to or from the work site. Make satisfactory and acceptable arrangements with the persons or agencies having jurisdiction over the damaged property concerning repair or replacement
- C. Brace and support existing pipes or conduits crossing the trench, or otherwise exposed to prevent trench settlement from disrupting the line or grade of the pipe or conduit. Before proceeding, the Contractor must reach an agreement with the Engineer on the method of bracing and support. Repair or replace all utility services broken or damaged at once to avoid inconvenience to customers. Storm sewers shall not be interrupted overnight. Use temporary arrangements, as approved by the Engineer, until any damaged items can be permanently repaired. Maintain all items damaged or destroyed by construction and subsequently repaired.
- D. Standard Detail 0201-0601-SD44 (attached) provides requirements for repair or replacement of sanitary or storm drains removed or damaged during installation of the water main.

3.03 RELOCATION

A. Relocate existing utilities or structures, where necessary, and restore it to a condition equal to that of the original facility. Obtain approval of the owner of the utility or structure prior to relocating and/or restoring the facility.

3.04 SEPARATION OF WATER MAINS AND SANITARY SEWERS

- A. General Consider the following factors when determining adequate separation:
 - 1. Materials and type of joints and restraints for water and sanitary sewer pipes.
 - 2. Soil conditions & backfill materials.
 - 3. Service and branch connections into the water main and sanitary sewer line.
 - 4. Compensating variations in horizontal and vertical separations.
 - 5. Space for repair and alterations of water and sanitary sewer pipes.
 - 6. Off-setting of pipes around manholes.

B. Parallel Installation

Lay water mains at least 10 feet horizontally from any existing or proposed sanitary sewer. Measure the distance from edge to edge. In cases where it is not practical to maintain a 10-foot separation, the applicable State Agency may allow deviation on a case-by-case basis, if supported by data from the Engineer. Such deviation may allow installation of the water main closer to a sanitary sewer, provided that the water main is laid in a separate trench or on an undisturbed earth shelf located on one side of the sanitary sewer at such an elevation that the bottom of the water main is at least 18 inches above the top of the sanitary sewer.

C. Crossings

Whenever water mains must cross sanitary sewer laterals or sanitary sewers, lay the water main at such an elevation that the bottom of the water main is 18 inches above the top of the sanitary sewer pipe. Maintain this vertical separation for the portion of the water main located within 10 feet horizontally of any sanitary sewer it crosses. The 10 feet is measured as a perpendicular distance from sanitary sewer line to the water line.

D. Exception

Notify the Engineer when it is impossible to obtain the proper horizontal and vertical separation as stipulated above. If directed by the Engineer, both the water main and sanitary sewer line shall be constructed of, mechanical joint ductile iron or welded joint protected steel pipe. Other types of restrained joints of equal or greater integrity may be used at the discretion of the Engineer after consultation with the applicable State Agency. Thermoplastic sanitary sewer pipe may be used provided mechanical or solvent weld pipe joints are used and accepted by the Engineer. Pressure test these joints before backfilling to assure that they are watertight. Where water mains must cross under a sanitary sewer, additional protection shall be provided by:

- 1. A vertical separation of at least 18 inches between the bottom of the sanitary sewer and the top of the water line.
- 2. Adequate structural support for the sanitary sewer to prevent excessive deflection of the joints and the settling on and breaking of the water line.
- 3. Centering the section of water pipe at the point of the crossing so that the joints shall be equidistant and as far as possible from the sanitary sewer line.

Consult the applicable State Agency, through the Engineer, to discuss the use of double casing or concrete encasement of sanitary sewer and/or water lines as possible alternatives when the above conditions cannot be met.

3.05 SEPARATION OF WATER MAINS AND STORM SEWERS

Where water mains and storm sewers would run parallel, lay water mains at least 10 feet horizontally from the existing or proposed storm sewer (measured from edge to edge). Where storm sewers and water mains would cross, place water mains at least 12 inches from the storm sewer (measured from edge to edge). In cases where it is not practical to maintain the specified separation, the Engineer may allow deviation on a case by case basis or as clearly called out in the plans. If the Engineer deems that such deviation will be allowed, install the water main as directed by the Engineer in such a way that does not compromise more stringent and desired separation from sanitary sewers per subsection 3.04.

END OF SECTION

CLEARING AND GRUBBING

PART 1: GENERAL

1.01 **PROTECTION**

Protect existing trees, shrubs and bushes located outside the clearing limits from damage for the life of this Contract.

1.02 REQUIREMENTS OF REGULATORY AGENCIES

Comply with State and local code requirements when disposing of trees, shrubs and all other materials removed under this Specification Section.

1.03 DISPOSAL FEES

Bear all expenses to obtain a suitable disposal area, haul to the disposal area, pay disposal fees, and dump at the disposal area.

PART 2: PRODUCTS

2.01 MATERIALS AND EQUIPMENT

Provide all materials and equipment required to complete all clearing and grubbing in accordance with this Specification Section.

PART 3: EXECUTION

3.01 CLEARING AND GRUBBING

Clear and grub the minimum area required to provide space for construction operations.

- A. Clear and grub the work site within easement and/or clearing limit lines shown on the Drawings or as shown elsewhere in the Contract Documents. Remove those items that are designated for removal or obstruct construction. This includes, but is not limited to: trees, downed timber, shrubs, bushes, vines, roots, stumps, undergrowth, rubbish, paving materials, debris, and all other objectionable materials. Site objects outside clearing limits shall not be removed. Only those portions of the construction area which are absolutely necessary and essential for construction shall be cleared. Minimize the length of time of ground disturbance as much as practical, especially within environmentally sensitive areas. Ground shall not be cleared and grubbed until immediately prior to construction.
- B. Notify the Engineer of locations where additional trees and shrubs will interfere with installation of facilities. Do not remove additional trees or shrubs without written permission of Engineer. Conduct operations to minimize disturbance of trees and shrubs. Trim trees and roots in accordance with the best horticultural practices, including sealing cuts to preserve the tree.

3.02 CLEARING (IMPROVED AREA)

- A. Remove site improvement objects such as signs, lawn ornaments, etc. which interfere with construction. Removed site improvement objects shall be stored in a manner protecting objects for reinstallation after construction is complete. Relocate the mailbox as necessary. Provide temporary traffic control signs when permanent signs are removed for construction. Temporary signs shall be worded to match permanent signs, except as necessary to be compatible with construction operations.
- B. Remove pavement, curb, and sidewalk in accordance with applicable State Standards for Road and Bridge Construction and as specified in these Contract Documents. Saw cuts may be eliminated where paving abuts curb or roadway expansion joints or construction joints, and pavement can be removed without damaging or disturbing curbs or remaining pavement. Remove sidewalks in full squares only. Saw cut sidewalks if no true joint exists.

3.03 DISPOSAL

- A. Burning of logs, stumps, roots, cuttings, and other material on the site will not be permitted.
- B. All materials obtained as a result of the clearing and grubbing operations shall be disposed of in accordance with the requirements of the applicable governing agencies.
- C. Chipping of brush materials will be permitted. However, Contractor shall bear all costs to dispose of the resultant chips at an approved location.

END OF SECTION

TRENCHING, BACKFILLING AND COMPACTING

PART 1: GENERAL

1.01 DEFINITIONS

- A. See Detail SD-53, SD-54, SD-55, SD-56, SD-57.
- B. Also refer to VDOT Asphalt Pavement Restoration Detail for Open Cut Utility Installation

1.02 SUBMITTALS

- A. All backfill materials (to be used for backfill, haunching, and bedding depending on local requirements), including common fill and selected fill [³/₄-inch clean granular fill, ³/₄-inch modified stone, ³/₄-inch minus granular fill, sand, ³/₈-inch crushed wash rock, ¹/₂-inch wet smooth stone, or ¹/₂-inch pug mix] shall be approved by the Engineer prior to placing the materials in the pipe trench. Test all backfill materials, whether obtained from the trench excavation or from an offsite source, as directed by the Engineer.
- B. All backfill materials must be approved by the Engineer before they are placed in the pipe trench. Submit samples of the materials to an approved testing agency for analysis as required by the Engineer. Submit the testing agency's test results and report to the Engineer. The report must state that the materials meet the requirements of these Specifications and the Specifications of Federal, State and local authorities (where applicable). Provide flowable fill in areas where it is required by the local street regulator, where the trench is subject to mine drainage and other areas specified in the drawings.

1.03 PROFILES AND TOPOGRAPHY

- A. Contours, topography, and profiles of the ground shown on the Drawings are believed to be reasonable approximations and are not guaranteed.
- B. The Contractor accepts the construction site with the conditions that existed at the time of bidding.

PART 2: PRODUCTS

2.01 COMMON FILL

- A. Common Fill shall be earth materials entirely free of: vegetation; trash; lumber; and frozen, soft or organic materials. No stones or rocks larger than the sizes listed below will be permitted in the Common Fill:
 - 1. Common Fill-Type A: No stones or rocks larger than 1-inch.
 - 2. Common Fill-Type B: No stones or rocks larger than 4-inches (measured longest dimension). At the discretion of the Engineer and depending upon the quality of the material, stones and rocks up to a maximum of 6 inches may be allowed on the area one foot above the pipe.

- B. Common fill material may be obtained from the trench excavation provided it has been tested in accordance with the requirements of Specification Section 02210.1.01 above and approved by the Engineer. Furnish the necessary approved common fill materials from an off-site source whenever approved material obtained from the trench excavation is insufficient to complete the backfill.
- C. The use of common fill is permitted in some circumstances as initial backfill for HDPE pipe; however, the size of stone and rock for backfill is limited in accordance with the pipe diameter. The maximum stone or rock size is limited to $\frac{1}{2}$ inches for pipes up to 4-inch diameter, $\frac{3}{4}$ inches for pipes 6- to 8-inch diameter, 1 inch for pipes 10- to 16-inch diameter and $1-\frac{1}{2}$ inches for larger pipes.

2.02 HAUNCHING FILL

A. Materials used for haunching around the pipe shall be selected aggregate backfill material, Size No. 21A, in accordance with latest VDOT Road and Bridge Specifications.

2.03 BEDDING FILL

Bedding fill materials vary from state to state, see special conditions and detail drawings for the appropriate materials for local use.

- A. <u>¾ inch clean granular fill material</u> shall meet the sieve analysis requirements of AASHTO as follows 1-inch sieve passing 100%, ½-inch sieve passing 0-5% and sieve size No. 4 passing 0-1%. This material may be wrapped in filter fabric (trench bottom, side, and over top of clean granular fill), as directed by the Engineer, to prevent the migration of finer grained soils into this material or the migration of this material into the trench bottom or sidewall.
- B. <u>³/₄ inch Minus or Modified granular fill material</u> contains additional fine material and may be used as noted in specific pipe specifications. Material shall meet the sieve analysis requirements of AASHTO as follows 1-inch sieve passing 100%, ³/₄-inch sieve passing 80-90%, No. 4 sieve passing 25-50%, No. 10 sieve passing 0-20% No. 200 passing sieve 0-5%.
- C. VDOT Crush Run stone #25 or #26, in accordance with latest VDOT Road and Bridge Specifications
- D. AASHTO #57 coarse aggregate stone.
- E. Not Used.
- F. Not Used.

2.04 FILTER FABRIC

A. Filter fabric shall be non-woven, synthetic fiber material with sieve design to prevent the select material in the pipe bedding and haunching from migrating into

the surrounding soils. The material shall have a minimum: thickness of 15 mils, tensile strength of 130 lbs., elongation at break of 64%, and trapezoidal tear strength of 70 lbs.

2.05 FLOWABLE FILL

A. Flowable fill is suitable for use as backfilling for utility trenches. The basic requirements for furnishing, mixing, and transporting flowable fill are as follows. Materials shall conform to the following standards: Cement ASTM C150, Fly Ash ASTM C618, Class C or Class F. Fine Aggregate shall be natural or manufactured sand, or a combination thereof, free from injurious amounts of salt, alkali, vegetable matter or other objectionable material. It is intended that the fine aggregate be fine enough to stay in suspension in the mortar to the extent required for proper flow. The fine aggregate shall conform to the following gradation:

Sieve Size	% Passing		
3/4 inch	100		
No. 200	0-10		

If a flowable mixture cannot be produced, the sand may be rejected.

- B. The following are given as typical mix designs for trial mixes. Adjustments of the proportions may be made to achieve proper solid suspension and optimum flowability. Admixtures may be used if desired to improve the characteristics of the mix. The suggested quantities of dry material per cubic yard are as follows:
 - 1. **Option 1** Cement 50 lbs, Fly Ash 250 lbs. Fine Aggregate 2910 lbs., Water approximately 60 gallons
 - 2. **Option 2** Cement 100 lbs. Fly Ash 250 lbs, Fine Aggregate 2800 lbs., Water approximately 60 gallons
 - 3. **Option 3** Cement 100 lbs., Fly Ash 300 lbs., Fine aggregate 2600 lbs., Water approximately 70 gallons
- C. Consistency may be tested by filling an open-minded 3-inch diameter cylinder 6inches high to the top with flowable fill. The cylinder shall be immediately pulled straight up and the correct consistency of the flowable fill shall produce a minimum 8-inch diameter circular-type spread with no segregation.

Materials are to be measured by weight and/or volumetric methods. The flowable fill may be mixed in a central concrete mixer, a ready-mix truck, or by other acceptable methods. The flowable fill shall be transported to the point of placement in a revolving drum mixer or in an agitator unit.

D. <u>Ductile Iron Pipe in Soil:</u> Soil shall be coarse to fine, sandy natural soil material with maximum stone size of 1-inch and shall meet ASTM D2487 "Standard Method for Classification of Soils for Engineering Purposes". Scarify 2 inches deep before placing pipe.

PART 3: EXECUTION

3.01 CONSTRUCTION EQUIPMENT

All backfilling and materials handling equipment shall have rubber tires when mains are located in or adjacent to pavements. Crawler equipment shall be permitted when there is no danger of damaging pavement. It is the Contractor's responsibility, to repair, at their expense, any damages due to the use of any equipment to complete the work.

3.02 NOISE, DUST AND ODOR CONTROL

Conduct all construction activities so as to eliminate all unnecessary noise, dust and odors.

3.03 PROTECTION OF TREES

Take special care to avoid damage to trees and their root system. Open trenching shall not be used for established trees in areas marked on the plans and designated 'Root Protection Zone'. In these areas, methods to be used include tunneling or boring. In other areas where established trees are to remain with roots in the path of the trench line, the Engineer shall direct acceptable means to install pipe through tree roots. In these areas, methods to be used careful cutting (not ripping or tearing) of larger tree roots. In all cases, operate equipment within the limb spread in a manner which will not injure trees, trunks, branches or their roots. Pay particular attention when employing booms, storing materials, and handling excavated materials.

3.04 TRENCH SUPPORT

Support open cut excavation for mains where trenching may cause danger to life, unnecessary damage to street pavement, trees, structures, poles, utilities, or other private or public property. Support the sides of the excavation by adequate and suitable sheeting, shoring, bracing or other approved means in accordance with all applicable Federal, State, County, Municipal and OSHA rules and regulations during the progress of the work, whenever and wherever it is necessary. Maintain the trench support materials and equipment in place until backfilling operations have progressed to the point where the supports may be withdrawn without endangering life or property per Article 6 on safety issues.

3.05 TRENCH EXCAVATION AND BOTTOM PREPARATION

A. <u>General Excavation</u>

General excavation shall consist of the satisfactory removal and disposal of all material taken from within the limits of the Work contracted, meaning the material lying between the original ground line and the finished ground line as shown on the Drawings regardless of whether the original ground line is exposed to air or is covered by water. Excavation below existing ground line to enable any required construction or removals is included. It is distinctly understood that any reference to earth, rock, silt, debris or other materials on the Drawings or in the Specifications is solely for the Owner's information and shall not be taken as an indication of classified excavation or the quantity of earth, rock, silt, debris or other material encountered.

Excavation to the lines and grades indicated on the Drawings or established in the field by the Engineer. Backfill over-excavated areas with approved fill material. All labor and materials shall be furnished at the Contractor's expense.

Keep all excavations free from water. Maintain groundwater a minimum of 6inches below excavations. Remove soil which is disturbed by pressure or flow of groundwater and replace with free draining material.

Remove pavement over excavations made in paved roadways by saw cutting, milling, or removal by a trench machine. Cut the full depth of the pavement with straight lines and squared edges.

Dispose of excess excavated materials and excavated materials unsuitable for backfilling off site. Furnish the Engineer with satisfactory evidence that an appropriate disposal site was used.

B. Rock Excavation

If the Contract includes a unit price for rock excavation, it includes the removal, hauling, stockpiling and/or proper disposal the rock per the Specification Section 01075 Basis of Payment. Rock is defined as:

- 1. boulders or loose rock having a volume of one cubic yard or more;
- material which cannot be loosened or broken down by ripping with a hydraulic ripper or other Engineer approved devices and equipment designed to remove rock; or
- 3. material that requires systematic blasting, backhoe ramming, barring, or wedging for removal.

Notify the Engineer promptly upon encountering rock. The Engineer's determination as to whether the material meets the definition of rock and Engineer's measurement of the volume of rock removal for which the Contractor is entitled to payment will be final and conclusive. No payment will be made for rock removed without Engineer's approval.

Strip rock for measurements as directed by the Engineer. No payment will be made for rock excavated or loosened before measurement. Only rock actually removed will be paid for, and in no case will payment be made for rock removal beyond the payment limits shown for a standard trench or more than 12 inches beyond the edge of a pipeline or 6 inches below its bottom for pipes of nominal OD 24 inches and less, unless such rock has been removed at the direction of Engineer.

C. Blasting Rock

Blasting is not allowed unless expressly permitted by the Engineer. Notify the Engineer in advance of blasting activity. Provide evidence to the Engineer that the proposed blasting will comply fully with Laws or Regulations.

Do not blast where limited or prohibited by any Federal, State or local laws or regulations, or in violation of any limitation or restriction contained in any right-ofway, or wherever specifically prohibited in any Drawing or other Contract Document. Do not blast within forty (40) feet of any pipe or structure without specific permission from the Owner. Properly cover blasts and protect the pipe or structure. Warn all persons in the vicinity. Blasting shall be at the risk of the Contractor who shall be liable for all damages to persons or property. Secure and pay for all necessary permits. Perform whatever pre-blast surveys and investigations that may be required by the circumstances and/or by Federal, State or local laws.

Prepare a blasting plan and submit it to the Engineer for approval prior to commencing any blasting work. The plan shall state all procedures and methods which will be used to monitor and mitigate the effect or impact of the proposed blasting work.

Employ an experienced blaster holding a blasting license issued by the applicable State to carry out the blasting work. Use, handle, and store explosives as prescribed by the applicable state and federal regulations. Keep all explosives in a safe place at a sufficient distance from the Work so that, in case of accident, no damage will occur to any part of the Work. Contractor shall be held responsible for and shall pay for all damage caused by blasting operations or accidental explosion.

D. Trench Width

Widths of trenches shall be held to a minimum to accommodate the pipe and appurtenances. The trench width shall be measured at the top of the pipe barrel and shall conform to the following limits:

- 1. <u>Earth</u>
 - a. Minimum: Outside diameter of the pipe barrel plus 8-inches, i.e., 4-inches each side.
 - b. Maximum: Nominal pipe diameter plus 24-inches.
- 2. <u>Rock</u>
 - a. Minimum: Outside diameter of the pipe barrel plus 24-inches, i.e., 12-inches each side.
 - b. Maximum: Normal pipe diameter plus 30-inches (Contractor will only be compensated for the minimum described above.
- E. Excessive Trench Width

Provide additional backfill, haunching, and bedding material, as specified in Specification Sections 02210.2.01, 02210.2.02, and 02210.2.03 as approved by the engineer to fill any trench excavation that exceeds the maximum trench width defined in Specification Section 02110.3.05.D. Dispose of excess excavated materials off site at no cost to the Owner. Furnish the Engineer with satisfactory evidence that an appropriate disposal site was used.

F. Trench Depth

- 1. <u>General</u>: Provide prescribed minimum cover from the top of the pipe barrel to the top of the finished grade of the roadway, unless otherwise authorized by the Engineer, or as shown on the plans.
- 2. <u>Earth</u>: Excavate to the depth required, so as to provide a uniform and continuous bearing and support for the pipe barrel on solid and undisturbed ground at every point between joints. It will be permissible to disturb the finished trench bottom over a maximum length of 18 inches near the middle of each length of pipe by the withdrawal of pipe slings or other lifting tackle. Provide bell holes. Prepare the finished trench bottom accurately using hand tools.
- 3. <u>Rock:</u> Excavate trenches in rock or boulders 6-inches below the pipe barrel for pipe 24-inches or less in diameter. Remove all loose material from the trench bottom. Prepare a pipe bed using bedding material as specified in Specification Section 02210.2.03.
- 4. <u>Unsuitable Bottom</u>: Notify the Engineer whenever unsuitable material is found below subgrade. Remove the material over the area and to the depth determined by the Engineer. Provide compacted bedding material as specified in Specification Section 02210.2.03 to restore the trench bottom to the required grade in these areas.

G. Open Trench Length

The length or size of excavation shall be controlled by the particular surrounding conditions but shall always be confined to the limits prescribed by Engineer. If the excavation becomes a hazard, or if it excessively restricts traffic at any point, Engineer may require special construction procedures such as limiting the length of the open trench or prohibiting stacking excavated material in the street. Take precautions to prevent injury to the public due to open trenches. All trenches, excavated material, equipment, or other obstacles which could be dangerous to the public, shall be well lighted.

3.06 TRENCH BACKFILLING - OPEN TERRAIN

All trench backfilling shall be compacted so that no settlement occurs and is stable with surrounding soil that also shall not have settled.

- A. Ductile Iron Pipe and HDPE Pipe
 - 1. <u>Bedding</u>
 - a. <u>In Suitable Soil</u> See Section 2.03 for definition of soil and means of bedding.
 - b. <u>In Rock or Unsuitable Soil</u> When encountering rock or unsuitable material, prepare pipe bedding immediately before pipe is laid. In this instance, compact clean granular fill as described in Specification Section 02210.2.03 from 6 inches below the pipe to the bottom of the pipe.

- 2. <u>Haunching:</u> Place haunching from the bottom of the pipe barrel to the centerline (springline) of the pipe barrel with Haunching Fill (Section 2.02) or clean, granular fill as described in Specification Sections 2.03. See Drawings for required haunching material. Take care to avoid injuring or moving the pipe. Place the material in uniform 6- to 12-inch loose layers and compact each layer so as to eliminate the possibility of settlement, pipe misalignment, or damage of joints.
- 3. <u>Initial Trench Backfill:</u> Backfill from the centerline (springline) of the pipe barrel to 12 inches above the pipe with Haunching Fill (Section 2.02) or clean, granular fill as described in Specification 2.03. See Drawings for required initial trench backfill material. Mechanical equipment may be used to place the backfill. Place the material in such a manner that the material does not free fall, but rather flows onto the previously placed material. Consolidate the backfill in such a manner as will ensure the minimum possible settlement and the least interference with traffic. Do not compact the backfill with mechanical equipment, such as wheeled vehicles, unless sufficient cover is provided over the pipe to prevent damage to the pipe.
- 4. <u>Final Trench Backfill</u>: Backfill trench from 12-inches above the pipe to final grade withCommon Fill-Type B, as described in Specification Section 02210.2.01. Mechanical equipment may be used to place the backfill. Place the material in such a manner that the material does not free fall, but rather flows onto the previously placed material. Consolidate the backfill in such a manner as will ensure the minimum possible settlement and the least interference with traffic. Do not compact the backfill with mechanical equipment, such as wheeled vehicles, unless sufficient cover is provided over the pipe to prevent damage to the pipe.
- 5. <u>Surface Conditions:</u> Attend to the trench surface regularly during the course of the Contract. Take prompt corrective measures to correct any settlement or wash-out. Maintain the trench surface in a safe condition that does not interfere with natural drainage.
- 6. <u>Deficiency of Backfill:</u> Any material required for backfilling the trenches or for filling depressions caused by settlement or wash-out shall be supplied and placed by the Contractor at his expense.

B. PVC

- <u>Bedding</u>: Prepare pipe bedding immediately before pipe is laid. Use compacted clean, granular fill as described in Specification Section 2.03 (a) or (b) from 6 inches below the pipe to the bottom of the pipe.
- 2. <u>Haunching and Initial Backfill</u>: Place haunching and initial backfill from the bottom of the pipe barrel to 12 inches above the top of the pipe barrel with Haunching Fill (Section 2.02) or clean, granular fill as described in Specification Section 02210.2.03. When material with high void ratios (e.g. ³/₄ inch clean granular fill) are used for embedment, it is possible for fines in the trench walls to migrate into the voids. This can cause some loss of support. An alternative method is to install filter fabric in the

boundary between the trench and the fill to prevent migration. Place the clean granular material in uniform 6- to 12-inch loose layers and compact each layer so as to eliminate the possibility of settlement, pipe misalignment, or damage of joints. Another alternative is to use materials containing fines, (e.g. ³/₄ inch minus or modified).

- 3. <u>Remaining Trench Backfill</u>: Backfill from 12-inches above the pipe to finished grade with Common Fill-Type B, as described in Specification Section 02210.2.01. Mechanical equipment may be used to place the backfill. Place the material in such a manner that the material does not free fall, but rather flows onto the previously placed material. Consolidate the backfill in such a manner as will ensure the minimum possible settlement and the least interference with traffic. Do not compact the backfill with mechanical equipment, such as wheeled vehicles, unless sufficient cover is provided over the pipe to prevent damage to the pipe.
- 4. <u>Surface Conditions</u>: Attend to the trench surface regularly during the course of the Contract. Take prompt corrective measures to correct any settlement or wash-out. Maintain the trench surface in a safe condition that does not interfere with natural drainage.
- 5. <u>Deficiency of Backfill</u>: Any material required for backfilling the trenches or for filling depressions caused by settlement or wash-out shall be supplied and placed by the Contractor at his expense.

3.07 TRENCH BACKFILLING – Under or Within 18 inches of Driveways and Roads

A. <u>Bedding</u>

Ductile Iron Pipe and HDPE Pipe: Install bedding for selected pipe material in accordance with Section 2.03, from 6 inches below the pipe to the bottom of the pipe.

PVC Pipe: Install bedding for selected pipe material in accordance with Section 2.03 (a) or (b) from 6 inches below the pipe to the bottom of the pipe.

B. <u>Haunching and Backfill</u>

Haunch around the pipe and fill the remainder of the excavation using Haunching Fill (Section 2.02). Place the material in uniform 6- to 12-inch loose layers and compact each layer so as to eliminate the possibility of settlement, pipe misalignment, or damage of joints. Take care to avoid injuring or moving the pipe.

C. <u>Surface Conditions</u>

Attend to the trench surface regularly during the course of the Contract. Take prompt corrective measures to correct any settlement or wash-out. Maintain the trench surface in a safe condition that does not interfere with natural drainage.

D. Deficiency of Backfill

Any material required for backfilling the trenches or for filling depressions caused by settlement or wash-out shall be supplied and placed by the Contractor at his expense.

3.08 SPECIAL BACKFILLING (Under Roads – Option to the Contractor)

A. Bedding

Install bedding for selected pipe material in accordance with Section 2.03.

B. Haunching and Initial Backfill

Place haunching and initial backfill from the bottom of the pipe barrel to 12inches above the top of the pipe barrel with clean, granular fill as described in Specification Section 02210.2.03. When material with high void ratios (e.g. ³/₄ inch clean granular fill) are used for embedment, it is possible for fines in the trench walls to migrate into the voids. This can cause some loss of support. An alternative method is to install filter fabric in the boundary between the trench and the fill to prevent migration. Place the clean granular material in uniform 6- to 12inch loose layers and compact each layer so as to eliminate the possibility of settlement, pipe misalignment, or damage of joints. Another alternative is to use materials containing fines, (e.g. ³/₄ inch minus or modified).

C. Remaining Trench Backfill

Backfill from the top of the pipe to subgrade, all cuts, excavations, or other damage done to the public right-of-way with flowable fill as described below. Use flowable fill when required as a condition of the right-of-way excavation permit.

- 1. Flowable fill shall have the following characteristics:
 - a. Unconfined Compressive Strength (28 day) 50-150 psi.
 - b. Flow Test diameter of spread < 8 inches.
- 2. <u>Design</u>: Submit the mix design to the Engineer for approval. A trial batch demonstration may be required. The mix design shall include a list of all ingredients, the source of all materials, the gradation of all aggregates, the names of all admixtures and dosage rates, and the batch rates. Document and justify minor mix design changes, after the trial batch verification, prior to implementation. This does not include adjustments to compensate for routine moisture fluctuations. Resubmit the mix design for approval of changes in the source of materials, the addition or deletion of admixtures, or changes in cementitious materials. The Contractor may be required to provide test data from a laboratory, inspected by the Cement and Concrete Reference Laboratory and approved by the Municipality, which shows the proposed mix design is in accordance with the requirements listed above.
- 3. <u>Flow Test</u>: Place a 3-inch diameter by 6-inch high open-ended cylinder on a smooth, nonporous, level surface and fill it to the top with the flowable fill. Pull the cylinder straight up within 5 seconds of filling.

Measure the spread of the fill. The minimum diameter of the spread shall be 8-inches.

- 4. <u>Placement</u>: Discharge the mixture from the mixing equipment into the space to be filled by a reasonable means. The flowable fill shall be brought up uniformly to the fill line. Each filling stage shall be as continuous as practicable. Do not place concrete on the flowable fill until all bleeding water has disappeared and the resistance, as measured by ASTM C403, is at least 60 psi, or as directed by Engineer. Do not place asphalt until at least 24 hours after the fill is completely in place.
- 5. <u>Limitations</u>: Do not place flowable fill on frozen ground. Protect flowable fill from freezing until the material has stiffened and bleeding water has disappeared. As the temperature nears freezing, additional curing time may be needed.
- D. Surface Conditions

Attend to the trench surface regularly during the course of the Contract. Take prompt corrective measures to correct any settlement or wash-out. Maintain the trench surface in a safe condition that does not interfere with natural drainage.

E. <u>Deficiency of Backfill</u>

Any material required for backfilling the trenches or for filling depressions caused by settlement or wash-out shall be supplied and placed by the Contractor at his expense.

3.09 QUALITY ASSURANCE TESTING

The Owner reserves the right to have the Contractor provide Independent Quality Assurance Testing for the backfill material, at the Contractor's expense.

3.10 TRENCH MAINTENANCE

Assume full responsibility for the condition of the trenches for a period of one (1) year from the date of the final acceptance of the Contractor's work, or as required by state, county or local authorities, and any materials required for filling depressions caused by settlement or wash-out shall be supplied and placed by the Contractor at their expense.

END OF SECTION

CASING INSTALLATION

PART 1: GENERAL

1.01 GENERAL REQUIREMENTS

The installation of casing pipe shall conform to these Specifications and any Federal, State or local Highway requirements or applicable Railroad requirements whichever may be more restrictive.

1.02 SUBMITTALS

Submit details of proposed jacking or boring pits to the Engineer showing locations, dimensions, and details of sheeting and shoring required, if requested.

1.03 RELATED WORK

Excavation, backfilling and compaction for jacking and receiving pits and for open cut installation shall conform to the requirements set forth in Specification Section 02210.

PART 2: PRODUCTS

2.01 MATERIAL

Casing pipe shall be bare wall steel pipe with a minimum yield strength of 35,000 psi and a minimum wall thickness as listed below:

Casing Outside	Highway Crossings	Railroad Crossings
Diameter	Casing Wall Thickness	Casing Wall Thickness
Inches	Inches	Inches
8.625	0.250	0.250
10.75	0.250	0.250
12.75	0.250	0.250
14	0.250	0.281
16	0.250	0.281
18	0.250	0.312
20	0.312	0.344
24	0.312	0.406
30	0.375	0.469
36	0.500	0.532
42	0.500	0.563
48	0.625	0.625
54	0.625	0.688
60	0.625	0.750
66	0.625	0.813
72	0.750	0.875

Smooth wall steel plates with a nominal diameter of over 54 inches shall not be permitted.

The inside diameter of the casing pipe shall be: at least four (4) inches greater than the outside diameter of the carrier pipe joints or couplings for carrier pipe less than six (6) inches in diameter; and at least six (6) inches greater than the outside diameter of the carrier pipe joints or couplings for carrier pipe six (6) inches and greater in diameter.

PART 3: EXECUTION

3.01 ALIGNMENT AND GRADE

Locate pipelines to cross roadways or tracks at approximately right angles where practicable, but preferably at not less than 45 degrees. Do not place pipelines in culverts or under bridges where there is a likelihood of their restricting the area required for the purposes for which the bridges or culverts were built, or of endangering the foundations. Install the casing pipe on an even grade for its entire length and sloped to one end or as noted in a profile plan if provided. Satisfy a maximum tolerance of 1.5% (18 inches in one hundred feet) with the desired location of the casing or as otherwise required by regulation or specified on the plans, whichever is more restrictive.

3.02 WELDING

Connect steel casing sections by welding. Welding shall conform to AWWA Standard C206.

3.03 PROTECTION AT ENDS OF CASING

Block up both ends of casings in such a way as to prevent the entrance of foreign material, but to allow leakage to pass in the event of a carrier break.

3.04 DEPTH OF INSTALLATION

Unless the depth of casing pipe is specifically specified on the drawings, the casing pipe depth shall be in accordance with highway or railroad requirements.

3.05 CASING INSULATORS

The carrier pipe and casing shall be separated by an insulator. The insulator spacing shall be installed to support the weight of the pipe and contents. As a minimum, an insulator shall be placed a maximum of 3 foot from each side of a joint and evenly spaced along the carrier pipe with 3 insulators per each length of carrier pipe. Timber skids are not allowed. Casing insulators shall be sized according to the manufacture's specifications for pipe sizes from the following list of approved manufactures and casing types.

- A. Cascade Water Works Manufacturing Company (Stainless Steel only).
- B. Pipeline Seal and Insulator, Inc. (Carbon Steel with polyvinyl chloride or the Ranger II model).
- C. Advanced Products and Systems, Inc. (Model SI).
- D. Power Seal Pipeline Products Corp. (Model 4810).

E. RACI (polyethylene model F-60 for 12-inch carrier pipe and smaller). RACI shall not be used for carrier pipe larger than 12-inch.

At the sole discretion of the Engineer, alternate manufactures in lieu of those described above and new or improved products by the same manufactures may be permitted. To seek approval, adequately describe any proposed alternate product and submit the same with shop drawings and specifications to the Engineer. The Contractor cannot proceed to employ said alternate products prior to receiving written approval of from the Engineer.

3.06 INSTALLATION

Refer to Standard Detail 0201-0601-SD45 at the end of this Specification Section for a typical casing installation detail.

Install casing pipes by one of the following methods:

A. Jacking

This method shall be in accordance with the current American Railway Engineering Association Specifications, Chapter 1, Part 4, "Jacking Culvert Pipe Through Fills", except that steel pipe shall be used with welded joints. Conduct this operation without hand mining ahead of the pipe and without the use of any type of boring, auguring or drilling equipment.

Design the bracing, backstops, and jacks so that the jacking can progress without stoppage (except for adding lengths of pipe).

B. Drilling

This method employs the use of an oil field type rock roller bit, or a plate bit made up of individual roller cutter units, welded to the pipe casing being installed. Turn the pipe for its entire length from the drilling machine to the head to give the bit the necessary cutting action against the ground being drilled. Inject high density slurry (oil field drilling mud) through a supply line to the head to act as a cutter lubricant. Inject this slurry at the rear of the cutter units to prevent any jetting action ahead of the pipe. Advance the drilling machine on a set of steel rails (thus advancing the pipe) by a set of hydraulic jacks. The method can be used to drill earth or rock.

C. Boring

This method consists of pushing the pipe into the fill with a boring auger rotating within the pipe to remove the soil. When augers or similar devices are used for pipe placement, the front of the pipe shall be provided with mechanical arrangements or devices that will positively prevent the auger and cutting head from leading the pipe so that there will be no unsupported excavation ahead of the pipe. The auger and cutting head arrangement shall be removable from within the pipe in the event an obstruction is encountered. The over-cut by the cutting head shall not exceed the outside diameter of the pipe by more than one-

half inch. The face of the cutting head shall be arranged to provide reasonable obstruction to the free flow of soft or poor material.

If an obstruction is encountered during installation that stops the forward action of the pipe, and if it becomes evident that it is impossible to advance the pipe, operations will cease, and the pipe shall be abandoned in place and filled completely with grout.

Bored or jacked installations shall have a bore hole essentially the same as the outside diameter of the pipe. Grout any voids that develop. Also grout around the casing pipe when the bore hole diameter is greater than the outside diameter of the pipe by more than 1 inch.

D. <u>Directional Drilling – see Specification 02458</u>

This process employs a drilling bit that is guided through soil to create a round cavity, which will stay intact with suitable soils and conditions for at least several days. Consequently, soil testing may be required by the Engineer. Test hole and ream as required. The drill head is propelled and remains linked to the rig by adding segments of rod as the head proceeds forward. After the hole has been completed the drill bit is removed and a pulling adaptor is attached to the drilling stem and pipe is secured to the adaptor.

As the adaptor is pulled back to the rig, segments of drill rod are removed. Pipe is either a continuous fused material or segments of restrained pipe are added as the adaptor is pulled back to the rig. The selection of pipe material and restraints, if required must be approved by the Engineer. The process continues until the adaptor returns to the rig and all of the water main is in place.

This process may be employed only if approved by Engineer and governing transportation and or regulating authority). The drilled opening and pipe inserted cannot be less than 3 inches in tolerance. Circulate grout in annular space completely. Alignment and grade must be maintained, and the drilled hole must be controllable using steering technology. Use radio equipment to track. Provide report of depth and location at 20-foot intervals during installation and submit as a report.

END OF SECTION

STREAM CROSSING

PART 1: GENERAL

1.01 SCOPE

Furnish all labor, materials, and equipment necessary to install the stream crossings as shown on the plans and described in the construction documents.

Install the stream crossings in such a manner as to protect the mains from erosion and to restore, as much as practicable, the stream banks and bottom to their original condition and in compliance with requirements of the regulating agency.

Protect the main from erosion by concrete encasement around the pipe or by a sufficient depth of compacted backfill as shown.

1.02 PROFILES AND TOPOGRAPHY

Contours, topography and profiles of the ground shown on the Drawings are believed to be reasonable approximations and are not guaranteed.

The Contractor accepts the construction site with the conditions that existed at the time of bidding.

1.03 RELATED WORK

Excavation, backfilling and compaction procedures shall conform to Specification Section 02210.

Concrete placement shall conform to Specification Section 03300.

PART 2: PRODUCTS

2.01 MATERIALS

The pipe shall be of special construction, having flexible watertight joints. Permanent taps and valves shall be provided at both ends of water crossings at a location not subject to flooding so that the section can be isolated for testing, locating of leaks, and repair in accordance with section 12VAC5-590-1180 of the Waterworks Regulations.

Excavation, fill, and concrete materials shall be as specified in Specification Sections 02210 and 03300.

PART 3: EXECUTION

3.01 CONSTRUCTION PROCEDURE

Special consideration. shall be given to the protection of the pipe from scour action, and in no event shall the minimum cover over the pipe be less than 2 feet below the scour level.

Comply with construction procedures if provided as a condition of the regulators stream opening permit. If methodology is not provided through permitting process, provide and submit the same to the Engineer and all Federal, State and local authorities having jurisdiction over the stream crossing for their review and approval.

3.02 STREAM BANK RESTORATION

Restore the stream banks by backfilling the main trench with mechanically compacted backfill of earth or rip rap, approved by the Engineer and in compliance with regulatory requirements, to the original ground surface (unless new contours are shown on drawings). The limits of compaction shall extend from the top of bank to top of bank on each side of the crossing as determined by the Engineer or as shown on the detail drawings provided.

Immediately following the completion of a stream crossing, place straw bales or siltfence along the trench excavation on each stream bank from within two (2) feet of the edge of water to beyond the limits of the excavated trench width per detail on straw bale and fabric fence. Straw bales or silt-fence shall remain in place until after the stream banks have been fine graded, fertilized and seeded, and the seeding has grown sufficiently to protect the stream banks from erosion.

3.03 STREAM BOTTOM RESTORATION

If the plans call for open cut across the stream bottom, backfill the trench within the stream bottom (high water to high water) mechanically compacted earth or riprap that has been approved by the Engineer and meeting regulatory requirements. Rip rap placement must be flush with stream bottoms from upstream to downstream.

END OF SECTION

BRIDGE CROSSING

PART 1: GENERAL

1.01 SCOPE OF WORK

Certain information regarding the reputed presence, size, character, and location of existing above ground and underground Facilities such as pipes, drains, sewers, electrical lines, telephone lines, cable TV lines, gas lines, and water lines has been shown on the Contract Drawings and/or provided in the contract documents. This information with respect to Underground Facilities is provided by the Owner in accordance with conditions described in the General Conditions and for information purposes only. Contractor is responsible to determine actual location of all utilities in proximity to the work for the purposes of the preparation of their bid and during construction.

1.02 NOTIFICATION OF UTILITIES

Notify the applicable State Agency with jurisdiction over the bridge facilities and all utility companies that construction work under this Contract will pass nearby containing their facilities. Notify these parties in advance to support the construction work (**minimum 72 hours**). All excavation in the vicinity of existing underground utilities shall be performed in accordance with applicable regulations.

1.03 BRIDGE CROSSINGS

Notify the applicable State Agency and Transportation Organization with jurisdiction over bridge facilities and/or all utility companies that construction work under this Contract will pass at or near the bridge structure. Notify these parties in advance to support the construction work (minimum 72 hours or as required by the organization with jurisdiction). All construction in the vicinity of existing bridge structures shall be performed in accordance with applicable regulations.

PART 2: PRODUCTS

2.01 MATERIALS

Furnish all materials for temporary support, adequate protection, and maintenance of all underground and surface utility structures, supports, drains, sewer and other obstructions encountered in the progress of the work.

The pipe material to be used for bridge crossings shall be steel or ductile iron as called out in the plans and approved by the Engineer.

For bridge crossings using steel pipe, all steel pipe to be ASTM A53 Grade "B" submerged arc-welded black steel pipe with ½-inch wall thickness, beveled ends, 50 Mil Pritec (or approved equal) coated exterior, and unlined interior. All steel pipe to be cement lined with 5/16-inch cement mortar lining in accordance with AWWA C602. If

lining not installed at factory, in place lining to be performed by contractor or subcontractor approved by owner.

For bridge crossings using ductile iron pipe, all ductile iron pipe to be fully restrained meeting requirements provided in Section 15105 or 15106 as applicable. All ductile iron pipe to have factory installed cement in accordance with AWWA C110 or epoxy lining in accordance with AWWA C116.

PART 3: EXECUTION

3.01 OBSTRUCTIONS BY OTHER UTILITY STRUCTURES

Support, relocate, remove, or reconstruct existing utility structures such as conduits, ducts, pipes, branch connections to main sewers, or drains. The obstruction shall be permanently supported, relocated, removed or reconstructed where they obstruct the grade or alignment of the pipe. Contractor must do so in cooperation with the owners of such utility structures. Before proceeding, the Contractor must reach an agreement with the Engineer on the method to work around the obstruction.

No deviation shall be made from the required line or depth without the consent of the Engineer.

3.02 REPAIRS

- A. Repair or replace any damage to existing structures, work, materials, or equipment incurred by Contractor's operations.
- B. Repair all damage to streets, roads, curbs sidewalks, highways, shoulders, ditches, embankments, culverts, bridges, trees, shrubs or other public or private property caused by transporting equipment, materials or personnel to or from the work site. Make satisfactory and acceptable arrangements with the persons or agencies having jurisdiction over the damaged property concerning repair or replacement
- C. Brace and support existing pipes or conduits crossing the trench, or otherwise exposed to prevent trench settlement from disrupting the line or grade of the pipe or conduit. Before proceeding, the Contractor must reach an agreement with the Engineer on the method of bracing and support. Repair or replace all utility services broken or damaged at once to avoid inconvenience to customers. Storm sewers shall not be interrupted overnight. Use temporary arrangements, as approved by the Engineer, until any damaged items can be permanently repaired. Maintain all items damaged or destroyed by construction and subsequently repaired.
- D. Standard Detail 0201-0601-SD44 provides requirements for repair or replacement of sanitary or storm drains removed or damaged during installation of the water main.

3.03 RELOCATION

Relocate existing utilities or structures, where necessary, and restore it to a condition equal to that of the original facility. Obtain approval of the owner of the utility or structure prior to relocating and/or restoring the facility.

3.04 BRIDGE CROSSINGS

- A. Supply cement lined steel or ductile iron pipe, cement or epoxy lined ductile iron or steel pipe fittings, related hardware, equipment, and labor to install water main in a dedicated utility bay beneath the bridge deck. Supply and install all required steel bends from bridge utility bay to meet required alignments to proposed buried DIP.
- B. For steel pipe installation, weld on steel pipe with three (3) passes in accordance with AWWA Standard C206. Supply welded flanges at end(s) of steel pipe for transition from steel pipe to DIP, including all necessary nuts, bolts gaskets, and related hardware. Gaskets to be full faced 1/8-inch thick.
- C. For ductile iron pipe installation, provide at least one support per length of pipe (unless "long span" pipe is utilized). Use the appropriate pressure class of pipe to support the weight of the pipe and its contents. Provide proper lateral and vertical support is needed to prevent "snaking."
- D. If construction of bridge is proposed at the same time as main installation, coordinate all activities with Bridge Contractor and Governing Agency.
- E. Size, supply, and install all required pipe roller supports for attachment to bridge. (Maximum spacing between supports is 10 feet.) Submit shop drawings to owner for approval. If construction of bridge is proposed at the same time as main installation, coordinate installation of pipe roller supports with Bridge Contractor. Supply, install, and coordinate installation of steel sleeves in proposed abutment walls of bridge with Bridge Contractor.

END OF SECTION

<u>GABIONS</u>

PART 1: GENERAL

1.01 SCOPE

- A. The work shall consist of furnishing, assembling and installing rock filled wire mesh gabion baskets and mattresses.
- B. Gabions shall consist of rectangular wire mesh formed containers filled with rock. Gabions will conform to one of the following mesh types:
 - 1. <u>Woven Mesh</u>: Non-raveling double twisted hexagonal wire mesh, consisting of two wires twisted together in two 180 degree turns.
 - 2. <u>Welded Mesh</u>: Welded-wire mesh with a uniform square or rectangular pattern and a resistance weld at each intersection. The welded wire connections shall conform with the requirements of ASTM A1064, including wire smaller than W1.2 (0.124 inches); except that the welded connections shall have a minimum average shear strength of 70% and a minimum shear strength of 60% of the minimum ultimate tensile strength of the wire.
- C. Gabions shall be furnished as baskets or mattresses, as shown in the construction plans. Baskets have a height of 12 inches or greater. Mattresses have a thickness of 12 inches or less.

PART 2: PRODUCTS

2.01 WIRE

- A. Wire for fabrication and assembly shall be hot-dipped galvanized. The wire shall have a minimum tensile strength of 60,000 psi. Galvanized steel wire shall conform to ASTM A641, Class 3, Soft Temper.
- B. Spiral binders are the standard fastener for welded-mesh gabion baskets and mattresses, and shall be formed from wire meeting the same quality and coating thickness requirements as specified for the gabion baskets and mattresses. Alternate fasteners for use with wire mesh gabions, such as ring fasteners, shall be formed from wire meeting the same quality and coating thickness requirements as specified for the gabions.
- C. Gabion baskets or mattresses with PVC coating shall be interconnected using ring fasteners made of stainless steel or PVC-coated spiral fasteners. All fasteners shall meet the closing requirements of the gabion manufacturer.

2.02 DESIGN

Baskets and mattresses shall be fabricated within a dimension tolerance of plus or minus 5 percent, except that the mattress height shall be within 10 percent. Gabions shall be fabricated, assembled and installed in accordance with the nominal wire sizes and dimensions found in Tables 1 and 2.

Gabion F	Baskets Heig	Table 1 ht 12, 18, or :	36 Inches: L	ength as Spec	rified
Type of Wire	Mesh Size Inches	Wire Diameter Inches	PVC Coating Inches	Total Diameter Inches	Galvanized Coating Oz./SF
Woven Mesh	3 ¼ x 4 ½ 3 ¼ x 4 ½	0.118 0.105	None 0.02	0.118 0.145	0.80 0.80
Selvage		0.153 0.132	None 0.02	0.153 0.172	0.80 0.80
Lacing and Internal Connecting Wire		0.086	0.02	0.126	0.70
Welded mesh	3 x 3 3 x 3	0.118 0.105	None 0.02	0.118 0.145	0.80 0.80
Spiral Binder		0.105	0.02	0.145	0.80

		Table 2			
Gabion Baskets Height 6, 9, or 12 Inches; Length as Specified					
Type of Wire	Mesh Size Inches	Wire Diameter Inches	PVC Coating Inches	Total Diameter Inches	Galvanized Coating Oz./SF
Woven Mesh	2 ½ x 3 ¼	0.086	0.02	0.126	0.80
Selvage		0.105	0.02	0.145	0.80
Lacing and Internal Connecting Wire		0.086	0.02	0.126	0.70
Welded mesh	1 ½ x 3	0.080	0.02	0.120	0.70
Spiral Binder		0.105	0.02	0.145	0.80

*NOTE: The wire sizes and PVC coating thickness shown are nominal sizes. The wire diameter includes the galvanizing coating thickness.

2.03 PVC COATING/PROTECTION

When Epoxy or Polyvinyl Chloride (PVC) coated wire is used, the galvanized wire shall be coated by fusion bonded epoxy; or fusion bonded, extruded, or extruded and bonded PVC material. The wire coating shall be colored black, gray, green or silvery; and the initial properties of the PVC coating shall meet the following requirements:

- A. <u>Specific Gravity</u>. In the range of 1.25 to 1.35, ASTM D792.
- B. <u>Abrasion Resistance</u>. The percentage of weight loss shall be less than 12%, when tested according to ASTM D1242, Method B at 200 cycles, CSI-A Abrader Tape, 80 Grit.
- C. <u>Brittleness Temperature</u>. Not higher than 15°F, ASTM D746.
- D. <u>Tensile Strength</u>. Extruded Coating (not less than 2,980 psi., ASTM D412). Fusion Bonded Coating (not less than 2,275 psi., ASTM D638).
- E. <u>Modulus of Elasticity</u>. Extruded Coating (not less than 2,700 psi. at 100 percent strain, ASTM D412). Fusion Bonded Coating (not less than 2050 psi. at 100 percent strain, ASTM D638).
- F. <u>Ultraviolet Light Exposure</u>. A test period of not less than 3000 hours, using apparatus Type E at 63°C, ASTM G23.
- G. Salt Spray Test. A test period of not less than 3000 hours, ASTM B117.

2.04 ROCK

A. Rock shall conform to the quality requirements in Wisconsin Construction Specification 9, Loose Rock Riprap, unless otherwise specified in the construction plan. At least 85 percent of the rock particles, by weight, shall be within the predominant rock size range shown in the table below.

Gabion Basket or Mattress Height	Predominant Rock Size Inches	Minimum Rock Dimension Inches	Maximum Rock Dimension Inches
18-or 36-Inch Basket	4 to 8	4	9
12-Inch Basket or Mattress	4 to 6	3	8
6-or 9-Inch Mattress	3 to 6	3	6

B. Prior to delivery to the site, the Contractor shall inform the Technician in writing of the source from which the rock will be obtained, and provide the test data by which the material was determined by the Contractor to meet the specification. Bedding or filter material, when specified, shall meet the gradation shown on the plans.

PART 3: EXECUTION

Unless otherwise specified in the construction plan, the assembly and placement of gabions shall be in accordance with the following procedures:

3.01 FOUNDATION PREPARATION

- A. The foundation on which the gabions are to be placed shall be cut or filled and graded to the lines and grades shown on the drawings. Surface irregularities, loose material, vegetation, and all foreign matter shall be removed from foundations. When fill is required, it shall consist of materials conforming to the specified requirements. Gabions and bedding or specified geotextiles shall not be placed until the foundation preparation is completed, and the subgrade surfaces have been inspected and approved by the Technician.
- B. Compaction of bedding or filter material will be required as specified in Wisconsin Construction Specification 8, Drainfill. The surface of the finished material shall be to grade and free of mounds, dips or windrows. Geotextile shall be installed in accordance with the requirements of Wisconsin Construction Specification 13, Geotextiles.

3.02 ASSEMBLY

- A. Rotate the gabion panels into position and join the vertical edges with fasteners for gabion assembly. Where lacing wire is used, wrap the wire with alternating single and double half-hitches at intervals between four (4) to five (5) inches.
- B. Where spiral fasteners are used for welded-wire mesh, crimp the ends to secure the spirals in place. Where ring type fasteners are used for basket assembly, install the fasteners at a maximum spacing of 6 inches. Use the same fastening procedures to install interior diaphragms where they are required.
- C. Interior diaphragms will be installed to assure that no open intervals are present that exceed three (3) feet.

3.03 PLACEMENT

- A. Place the empty gabions on the foundation and interconnect the adjacent gabions along the top, bottom, and vertical edges using lacing wire, spiral fasteners, or ring fasteners. Wrap the wire with alternating single and double half-hitches at intervals between four (4) to six (6) inches. Ring fasteners shall not be spaced more than six (6) inches apart. Spirals are screwed down at the connecting edges, then each end of the spiral is crimped to secure it in place.
- B Lacing wire will be used as needed to supplement the interconnection of welded mesh gabions, and the closing of lids. Interconnect each layer of gabions to the underlying layer of gabions along the front, back, and sides. Stagger the vertical joints between the gabions of adjacent rows and layers by at least one-half of a cell length.

3.02 FILLING

- A. After adjacent empty woven wire gabion units are set to line and grade and common sides properly connected, they shall be placed in straight line tension and stretched to remove any kinks from the mesh and to gain a uniform alignment. Staking of the gabions may be done to maintain the established proper alignment prior to the placement of rock. No stakes shall be placed through geotextile material.
- B. Internal connecting cross-tie wires shall be placed in each unrestrained gabion cell greater than 18 inches in height, including gabion cells left temporarily unrestrained. Two internal connecting wires shall be placed concurrently with rock placement, at each 12-inch interval of depth.
- C. In woven mesh gabions, these cross-ties will be placed evenly spaced along the front face and connecting to the back face. All cross-tie wires shall be looped around two mesh openings and each wire end shall be secured by a minimum of five 180 degree twists around itself after looping. In welded mesh gabions, these cross-ties or stiffeners will be placed across the corners of the gabions (at 12 inches from the corners) providing diagonal bracing. Preformed hooked wire stiffeners will be used.
- D. The gabions shall be carefully filled with rock, either by machine or hand methods, maintaining alignment, avoiding bulges, and providing a compact mass that minimizes voids. Machine placement will require supplementing with hand work to ensure the desired results. The cells in any row shall be filled in stages so that the depth of rock placed in any one cell does not exceed the depth of rock in any adjoining cell by more than 12 inches. Along the exposed faces, the outer layer of stone shall be carefully placed and arranged by hand to ensure a neat, compact placement with a uniform appearance.
- E. The last layer of rock shall be uniformly overfilled 1-2 inches for gabions and 0.5-1 inch for gabion mattresses to allow for rock settlement. Lids shall be stretched tight over the rock fill using only approved lid closing tools. The use of crowbars or other single point leverage bars for lid closing is prohibited. The lid shall be stretched until it meets the perimeter edges of the front and end panels. The gabion lid shall then be secured to the sides, ends, and diaphragms with spiral binders or lacing wire wrapped with alternating single and double half-hitches in the mesh openings. Ring fasteners spaced not more than six (6) inches apart may be used for lid closure.
- F. Any damage to the wire or coatings during assembly, placement and filling shall be repaired promptly in accordance with the manufacturer's recommendations or replaced with undamaged gabion baskets.

END OF SECTION

<u>SMALL-SCALE HORIZONTAL DIRECTIONAL DRILLING (HDD)</u> (Projects less than 250 feet or pipe size 12 inch and less)

PART 1: GENERAL

1.01 SCOPE OF WORK

A. Furnish all labor, materials, tools and equipment as necessary to construct a pipeline crossing by the horizontal directional drilling method. Furnish all labor, equipment, materials and supplies, and perform all work necessary to provide Owner with a complete, finished water main crossing. The finished work includes proper installation testing, restoration of underground utilities and environmental protection and restoration.

1.02 RELATED SECTIONS

Section 01300 – Submittals Section 02200 – Excavation, Backfilling and Compaction Section 15000 – Piping - General Provisions Section 15020 – Disinfecting Pipelines

1.03 QUALITY ASSURANCE

- A. The HDD equipment operator(s) shall be trained to operate the specific Horizontal Directional Drilling equipment for the Owner's project with at least 3 years experience in directional drilling obtained within the last five years that includes installation of potable water pipelines of the same or larger diameter and the same or greater lengths. All pipe and appurtenances of similar type and material shall be furnished by a single manufacturer.
- B. Perform HDD operations under the constant direction of a drilling supervisor who shall remain on site and be in responsible charge throughout the drilling operation. The Contractor's supervisor shall have supervised directional drilling of a minimum of 5,000 linear feet of pipe of a similar or greater diameter, of similar material, over similar lengths, and with similar subsurface conditions.
- C. Perform the work in conformance with the Directional Crossing Contractors Association (DCCA) published guidelines (latest edition) and pipe manufacturer's guidelines and recommendations.
- D. Adhere to the specifications; any changes must be expressly approved by the Engineer's. Approval of any aspect of any Directional Bore operation covered by this Specification shall in no way relieve the Contractor of its ultimate responsibility for the satisfactory completion of the work authorized under the Contract.

1.04 PROFILES AND TOPOGRAPHY

A. Contours, topography and profiles of the ground as may be shown on the Contract Drawings are believed to be reasonably correct, but are not guaranteed to be absolutely so and are presented only as an approximation. It is the Contractor's responsibility to verify all elevations required to successfully complete the crossing.

1.05 SUBMITTALS

- A. Prior to beginning work, submit to the Engineer copies of a report of schedules, calculations, procedures and any supplemental subsurface soil condition investigations performed along the path of the proposed crossing. The report will summarize the subsurface conditions that are known to the Contractor and that his proposed crossing procedure is based upon factual, best available information. If the subsurface conditions are known to the Contractor by previous work or geotechnical studies done in the immediate area, the information shall be recorded in the report along with any additional geotechnical studies performed by the Contractor. The report shall include the following:
 - 1. Subsurface Information
 - a. Record in the report subsurface conditions known to the Contractor by previous work or prior geotechnical studies performed in the immediate project area.
 - b. Boring information obtained by the Owner, if any, is listed in the Supplementary Conditions section of these Specifications.
 - c. Additional borings performed by the Contractor and analysis of soils along the path of the proposed crossing. The Contractor shall be responsible for obtaining and including in his bid price the cost of any additional borings along the pipe alignment which may be necessary to design the proposed directionally drilled crossing.

At a minimum any supplemental borings performed by the Contractor shall include standard classification of soils, standard penetration tests, split spoon sampling and sieve analysis. Test borings shall be performed to a minimum depth of ten (10) feet below the proposed pipe invert unless rock is encountered in which case test borings shall penetrate at least two feet into rock.

- 2. Drilling Equipment and Methods
 - a. Submit details of equipment and written procedure with working drawings describing the proposed boring method and the entire operation to be used. This shall include, but not be limited to, entry and exit pits; settlement pit; size, capacity and arrangement of drilling and pulling equipment; layout of carrier pipe; details and spacing of pipe rollers; type of current head; method of monitoring and controlling line and grade; method of detection of surface movement; and layout of any proposed construction staging areas.

- b. In addition, submit for approval nameplate data for the drilling equipment, mobile spoils removal unit, and Material Safety Data Sheets (MSDS) information for the drilling slurry compounds. This must be submitted and reviewed by the Engineer before work can proceed.
- 3. Piping

Submit shop drawings showing the pipe lengths, design details, joint details, etc. for the Engineer's review. Submittals shall include, but are not limited to, the following:

- a. All welding or fusion procedures to be used in fabrication of the different pipe materials and installation methods.
- b. Certified records for hydrostatic testing of all pipe materials to be used.
- c. An affidavit stating that all pipe materials furnished under this section have been manufactured in the United States of America and comply with all applicable provisions of referenced AWWA standards.
- 4. Proposed Alignment

Submit a graph in plan and profile plotting the pilot drilling hole alignment to the Engineer for review, including entry/exit angles and radius of curvature. After completion of the crossing, submit a final pipe alignment.

5. <u>Schedule</u>

Time schedule for completing the Directional Bore, including any delays due to anticipated soil conditions.

- 6. Calculations
 - a. Submit detailed design calculations for several representative loading conditions for the proposed crossing. If requested by the Engineer, submit calculations to support the design of any particular location of pipe anywhere along the length of the crossing at no additional cost to the Owner.
 - b. Design calculations shall be presented in a neat, readable format, with all figures, values and units included to facilitate ease of verification.
 - c. Calculations shall be submitted to demonstrate that the pipe thickness design is sufficient to meet all design criteria specified.
 - d. Calculations shall address the following loading conditions:

- Pre-installation: Hoop and longitudinal stress during hydrostatic test; spanning stress with pipe full of water and supported on installation rollers, and maximum roller / support spacing.
- ii) Installation/Post-Installation: Longitudinal stress from pulling force; longitudinal curvature stress at point of entry and in final position; external pressure from drilling fluid, overburden, and loads from the obstacle being crossed.
- iii) Post-Installation/In-Service: Hoop and longitudinal stress during hydrostatic test; internal working and surge pressure; buckling with internal vacuum.
- e. Perform and submit to the Engineer fluids pressure versus overburden strength calculations. These calculations shall be performed to determine minimum acceptable cover requirements and prevent drilling fluids breakout to the ground surface.
- f. All calculations shall bear the seal of a Registered Professional Engineer. Licensure in the State that the work is performed is preferred.

B. <u>Approval</u>

1. No work shall commence without approval by the Engineer. Details and design calculations shall be submitted and approved well in advance of the drilling operation to prevent delays in work. All final layout work, including grades, shall be the Contractor's responsibility.

1.06 JOB CONDITIONS

- A. <u>Any</u> nighttime work is <u>strictly regulated</u> and will be allowed only with prior approval granted by the Owner <u>subject to</u> regulatory agencies having jurisdiction. All crossing operations shall be accomplished during daylight hours, unless approved by the Engineer. Crossing work shall not begin after the hour pre-established as the latest starting time that will allow completion during daylight hours, unless approved by the engineer. The Contractor shall provide a Work Plan submittal indicating its proposed hours of operation and length of work week. All work plans shall be subject to compliance with all applicable regulatory requirements for construction activities and any off-site impacts.
- B. When hazards of night time work are carefully considered and determined to be insignificant, night time work may be allowed only to complete a properly planned crossing, and only if in the opinion of the Engineer the delay was caused by reasonably unavoidable circumstances, and that such night time work is necessary to avoid placing an undue economic hardship on the Contractor.
- C. In emergency situations, or where delay would increase the likelihood of a failure, nighttime work may be allowed to complete a delayed crossing.

D. All operations shall continue on a 24-hour per day basis during pipe pull back.

1.07 COORDINATION OF WORK

A. Coordinate connections to existing pipelines that require shutdown of Owner facilities. Owner will designate the time for these connections that could involve work during evenings, nights, Saturdays, Sundays, or holidays. Method of connection and designated times are to cause the least amount of disruption to Owner's water service to its customers. The cost for connections is to be included in the contract price. No contract price adjustment will be allowed for overtime, premium time, or other related costs.

1.08 USE OF EXISTING WATER SYSTEMS

- A. All use of existing water systems during construction by the Contractor shall be with the approval and direction of the system Owner and its representatives. The Contractor shall be responsible for all permits, fees, temporary piping, temporary meter rental/provisions, temporary backflow preventer rental/provision and other water utility requirements for supplying water during construction. Use the existing water system only at locations, times and conditions as set forth by the system owner or its representatives.
- B. If water is not readily available at the site or the Owner cannot provide the volume of flow required by the Contractor, provide potable water as needed from an off-site location at no additional cost to the Owner.

PART 2: PRODUCTS

2.01 PIPE

- A. Pipe shall be HDPE pipe with ductile iron pipe outside diameters in accordance with AWWA C906. Verify the appropriate dimension ratio based on the pipe, joint and material pull strength required for the directional drilling.
 - 1. HDPE pipe and related fittings shall be made with prime virgin resins exhibiting a minimum cell classification as defined in ASTM D3350 and meeting the PE 3408 code designation with maximum dimension ratios equal to 11.
 - 2. HDPE pipe 4-inch and larger nominal diameter shall be joined by means of zero leak-rate butt (thermal heat) fusion welds and/or approved flanged joints. Joints shall provide axial pullout resistance. Pipe shall meet the requirements of ANSI/AWWA C906, and have an outside diameter dimension of ductile iron pipe. Flanged joints shall not be used below finished grade for horizontal directional drilling applications.
 - 3. HDPE pipe shall have been continuously marked by the manufacturer with permanent printing indicating at a minimum the following.
 - a. Nominal size (inches);
 - b. Dimension ratio (DR);
- c. Pressure rating (psi);
- d. Trade name:
- e. Material classification (PE 3408);
- f. Plant, extruder and operator codes;
- g. Resin supplier code;
- h. Date produced; and
- i. HDPE pipe used for portable water mains shall bear the NSF Seal of Approval.
- 4. HDPE pipe shall be black in color with permanent blue colored stripes extruded into the pipe length or shall be solid blue color.
- 5. Installation Curvature: The pipeline curvature shall not have a radius less than as shown in Table 2458-1.

Table 2458-1. HDPE Pipe Deflection Information					
Pipe Diameter	Minimum Radius	Offset per 20-ft			
(inches)	of Curvature (feet)	Length (inches)			
4	23	9.3			
6	34	6.1			
8	44	4.6			
10	56	3.5			
12	67	3.0			

2.02 EQUIPMENT

A. General

All equipment for the Directional Bore shall have the capacity, stability, and necessary safety features required to fully comply with the specifications and requirements of this section without showing evidence of undue stress or failure. It shall be the responsibility of the Contractor to assure that the equipment to be used in the Directional Bore is in sound operating condition. Backup equipment shall be required in the event of an equipment breakdown and where the condition of the equipment to be used indicates that routine component replacement or repair will likely be necessary during the Directional Bore.

B. Directional Drilling System

The directional drilling system shall consist of over the road transportable field power unit, mud-mixing and recycling unit, a trailer or carriage-mounted drill unit, and all other support accessory vehicles and equipment. All system components shall be in sound operating condition with no broken welds, excessively worn parts, badly bent, or otherwise misaligned components. All drill pipe, reamers, pull back heads, swivels, drill heads and collars, pipe cradles, pipe rollers, ropes, cables, clamps, and other non-mechanical but essential items shall be in sound condition and replaced immediately when need is apparent. The equipment must be capable of drilling the specified length in a single bore.

1. Mud-Mixing and Recycle Units

The mud-mixing and recycle unit shall be a self-contained system designed to provide a supply of high-pressure bentonite based cutting fluid to the drill unit. It shall contain a fluid storage tank and a complete bentonite and drilling fluid additive(s) mixing system. The cutting fluid is to be mixed on site. The cutting fluid shall be formulated for this specific project and anticipated conditions. It shall permit changes to be made to the bentonite and drilling fluid additive(s) concentrations during drilling in response to changing soil conditions. The field power unit shall contain the power-taken off-driven high pressure cutting fluid pumping system. The recycle units shall be of a capacity to minimize the production of new cutting fluid and maximize the reuse and recirculation of original cutting fluid produced.

2. Directional Drill System

A carriage-mounted version of the drill system shall include a thrust frame. Both the trailer-mounted and carriage-mounted drill system shall be designed to rotate and push 10-foot (3-meter) minimum hollow drill sections into the tunnel being created by the boring head. The drill sections shall be made of high strength S-grade steel that permits them to bend to a 30-foot (9-meter) radius without yielding. Drill end fittings shall permit rapid makeup of the drill sections while meeting the torque, pressure and lineal load requirements of the system. The boring head itself shall be capable of housing a probe used by the Magnetic Guidance System (MGS) to determine tool depth and location from surface and to orient the head for steering. The MGS shall have a minimum accuracy of plus (+) or minus (-) two (2) percent of the vertical depth.

The drilling equipment must be fitted with a permanent alarm system capable of detecting an electric current. The system will have an audible alarm to warn the operator when the drill head nears electrified cables. The drilling equipment shall be grounded, protected, and operated in accordance with manufacturer's requirements for electric strike safety.

The control console shall contain a calibrated display of inclination, azimuth, tool face location, mud pump rates, and torque pressures. The downhole steering system accuracy shall be plus or minus one percent (\pm 1.0%) of the horizontal bore length such that the difference between actual depth and machine calculated depth is not more than 1 foot per hundred feet.

3. Restrictions

Other devices or utility placement systems for providing horizontal thrust other than those previously defined in the preceding sections shall not be used unless approved by the Engineer prior to commencement of the work. The proposed device or system will be evaluated prior to approval or rejection on its potential ability to complete the pipe placement satisfactorily without undue stoppage and to maintain line and grade within the tolerances prescribed by the particular condition of the project. Water sluicing methods, jetting with compressed air, or boring or tunneling devices with vibrating type heads that do not provide positive control of the line and grade shall not be allowed.

C. Spoils Equipment

The cutting fluid removal system shall include a self-contained vacuum truck which has sufficient vacuum and tank capacity to remove excess cutting fluid mixture and cuttings from the project site as required or directed by the Engineer. Spoils are not to be discharged into sewers or storm drains.

Contain all drilling and pipe lubricating mud by taking special measures to prevent run-off into adjacent properties and/or waterways. All surplus drilling and pipe lubricating mud will be removed from the site and properly disposed of by the Contractor. The Contractor will also be responsible for all required erosion control measures.

D. <u>Magnetic Guidance System</u>

A Magnetic Guidance System (MGS) probe and location of the drill head during the drilling operation. The tracker shall be capable of tracking at all depths up to one hundred feet and in any soil condition, including hard rock. It shall enable the driller to guide the drill head by providing immediate information on the tool face, azimuth (horizontal direction), and inclination (vertical direction). The tracker shall be accurate to +/-2% of the vertical depth of the borehole at sensing position at depths up to one hundred feet. Ferrous materials shall not influence or affect the MGS readings or accuracy.

Components: Supply all components and materials to install, operate, and maintain the MGS. This shall include, but not be limited to the following:

- 1. MGS Probe and Interface
- 2. Computer, Printer, and Software
- 3. DC Power Source, Current Control Box, and Coil/Tracking Wire.

The Magnetic Guidance System (MGS) shall be a Tensor TruTracker MGS, or other licensed and industry approved wire guidance system. The Engineer shall be advised of the unit to be used and is subject to his approval. Set up and operate the MGS using personnel experienced with this system. A Walk-over" tracking systems shall not be used, except as approved by the Engineer. Contractor shall provide Engineer with current calibration certification of MGS in accordance with manufacturer's specifications.

- E. If equipment breakdown or other unforeseen stoppages occur and forward motion of the directional cutting head is halted at any time other than for reasons planned in advance (addition of drill stems, etc.), the boring path shall be filled with a proper bentonite solution immediately, or as directed by the Engineer.
- F. The boring tool shall have steering capability and have an electronic tool detection system. The position of the tool during operation shall be capable of being determined accurately, horizontally within 1% of the horizontal distance of the borehole and vertically within 2% of the vertical depths of the borehole. The boring tool shall have a nominal steering radius of 9 meters (30 feet).

- G. The directional drilling machine shall consist of a hydraulically powered system to rotate, push and pull hollow drill pipe into the ground at a variable angle while delivering a pressurized fluid mixture to a guidable drill (bore) head. The machine shall be anchored to the ground to withstand the pulling, pushing and rotating pressure required to complete the crossing. The hydraulic power system shall be self-contained with sufficient pressure and volume to power drilling operations.
- H. Hydraulic system shall be free of leaks. Rig shall have a system to monitor and record maximum pullback pressure during pullback operations. The rig shall be grounded during drilling and pullback operations. There shall be a system to detect electrical current from the drilling string and an audible alarm that automatically sounds when an electrical current is detected.

2.03 DRILLING FLUIDS

- A. Drilling fluids shall consist of a mixture of potable water and gel-forming colloidal material, such as bentonite or a polymer surfactant mixture producing slurry of custard-like consistency.
- B. Where sandy or granular materials are encountered, a cement slurry or polymer supplement shall be considered for added strength and stability of the bore and over ream hole.
- C. No chemicals or polymer surfactant shall be used in the drilling fluid without written consent of the Engineer, and after a determination is made that the chemicals to be added are not harmful or corrosive to the facility and are environmentally safe. Clay must be totally inert and contain no risk to the environment.
- D. Provide Owner, Engineer and have on site at all times the Material Safety Data Sheets (MSDS) for all drilling compounds and chemicals.

2.04 TRACER WIRE

- A. Tracer or location wire shall be a direct burial #12 AWG Solid (.0808 inch diameter), steel core hard drawn extra high strength horizontal directional drill tracer wire, 1150# average tensile break load, 45 mil. High molecular weight-high density blue polyethylene jacket complying with ASTM D1248, 30-volt rating. The wire shall be contiguous except at test stations, valve boxes, and where splicing is required. All splices shall be encased with a 3M-Gel Pack model No. 054007-09053. Wire insulation shall be highly resistant to alkalis, acid and other destructive agents found in soil. Location Wire shall be from Copperhead Industries, LLC, part number 1230B-HS or approved equal.
- B. Tracer wire shall be installed simultaneously with pullback of the HDPE pipe. Wire shall either be wrapped around the pipe or taped to the pipe at 10-foot minimum intervals before installation.

PART 3: EXECUTION

3.01 SITE DISTURBANCE AND SOIL EROSION

- A. Sediment barriers shall be constructed as shown on the Drawings or where directed by the Engineer. All soil erosion and sediment control work shall be done in accordance with the Standards for Soil Erosion and Sediment Control for the location where the work is performed. Contractor shall maintain sediment barriers until the project is deemed complete.
- B. The Contractor shall be responsible for the preservation of all existing trees, plants, and other vegetation that are to remain within or adjacent to the construction site and shall also be responsible for protecting existing concrete curb, fence, utilities, and other structures that are located within or adjacent to the construction site.
- C. The Contractor assumes all liability for environmental damage and cleanup due to inadvertent discharges of slurry or other causes. Slurry materials shall be selected based on the soil conditions encountered to minimize the risk of mud returns.

3.02 PERSONNEL REQUIREMENTS:

- A. Provide a competent and experienced supervisor representing the Drilling Contractor who must be present at all times during actual operations. A responsible representative, who is thoroughly familiar with the equipment and type work to be performed, must be in direct charge and control of the operation at all times. In all cases the supervisor must be continually present at the job site during the actual Directional Pilot Hole, over reaming and pullback operations.
- B. Have a sufficient number of competent workers on the job at all times to ensure the Directional Bore is made in a timely and satisfactory manner. Adequate personnel for carrying out all phases of the actual Directional Bore operation must be on the job site at the beginning of work.
- C. If HDPE is specified for the carrier pipe, HDPE pipe thermal butt fusion welding is to be completed by a welder certified by the manufacturer of the pipe or pipe welding equipment, in accordance with the Plastic Pipe Institute "Handbook of Polyethylene Pipe," Polyethylene Joining Procedures, and 49 CFR 192, Subpart F, latest edition.
- D. If steel pipe is specified for the carrier or casing pipe, welding shall be performed by certified welders. The Contractor shall be responsible for the qualification of welders with qualification testing conducted by an independent testing agency in accordance with American Welding Society D1.1 requirements. Results of qualification testing shall be submitted to the Engineer for approval. Results of previous qualification tests performed within six months from the date of pipe installation will be acceptable. Results from qualification tests performed prior to six months from the date of pipe installation will not be acceptable. All costs associated with qualification testing shall be included in the unit prices bid.

- E. The Engineer and Owner must be notified 48 hours in advance of starting each phase of the work. The Directional Bore shall not begin until the Engineer is present at the job site and agrees that proper preparations for the operation have been made. The Engineer's approval for beginning the installation shall in no way relieve the Contractor of the ultimate responsibility for the satisfactory completion of the work as authorized under the Contract. It shall be the responsibility of Owner to provide inspection personnel at such times as appropriate without causing undue hardship by reason of delay to the Contractor.
- F. If the Contractor fails to begin the Directional Bore at the agreed time, the Owner will establish the next mutually convenient time to begin. To avoid undue hardship of either party, reasonable and mutual cooperation should be exercised where starting times are concerned. If one party fails to meet the agreed schedule, the other party is expected to consider a delayed start if the installation cannot be completed during daylight hours.

3.03 ALIGNMENT AND GRADE

- A. Determine and physically locate the depth, location, and size of all existing underground facilities in the vicinity of the proposed crossings and provide the Engineer with a comprehensive report of these facilities before starting any construction. The Contractor shall be held completely and solely responsible for any damages incurred. The kinds, locations and sizes of the existing underground utilities which may be shown on the Contract Drawings are intended only as a guide to the Contractor and are not guaranteed to be even approximately correct. Notify the owners of all existing utilities along the route and in the vicinity of the crossing prior to the construction to include all test borings and excavations.
- B. If utilities of unknown depth or other obstructions require grade or alignment deviations from the Plans, the grade and/or alignment may be adjusted with Engineer's approval. All adjustments shall permit gradual bends of the pipe to the original alignment beyond the directional bore section. At unusual site conditions, the Contractor may request a review of site conditions by the Engineer for additional adjustment, and such determination shall be final. An adjustment in alignment, position, or elevation approved by Engineer shall not be cause for an adjustment of costs.
- C. Pipe entry and exit points are to be allowed no more than five (5) feet of deviation from the staked centerline. The entry point may be moved up to twenty-five (25) feet further from the original entry point only with Engineer's approval. Exit point lengths greater than twenty-five (25) feet from the original point require Engineer's approval. Entry and exit points normally will not be allowed closer to the banks of a waterway being crossed. Any installation that deviates from the plan may be rejected and any rejected installation shall be reconstructed at the Contractor's expense.
- D. The vertical profile as shown on the drawings is the minimum depth to which the pipeline shall be installed. Contractor may, at his option and with the permission of Owner, elect to install the pipe at a greater depth than shown on the drawings, at no additional cost to the Owner.

3.04 INSTALLATION

- A. The Contractor shall be responsible for providing a Maintenance of Traffic Plan to the Engineer and local traffic law enforcement agency for review. The Maintenance of Traffic Plan shall show the location of all barricades, signs, devices and alternate routes for local traffic and pedestrian safety. Erection of the appropriate safety and warning devices in accordance with the USDOT "Manual of Uniform Traffic Control Devices" (MUTCD) shall be completed prior to beginning work and maintained until all construction is completed and the site restored.
- B. Specifically note in the Maintenance of Traffic Plan street intersections that are to remain open as required during the pipe pull-back operation, or traffic detours implemented. Install a temporary sleeve across the street intersections through which the pipe can be pulled or to construct a temporary bridge for the pipe over the intersections as required. No additional payment will be made for temporary structures required in order to permit access through street intersections or the implementation of traffic detours.
- C. The cost of restoring pavement, curb, sidewalk, driveways, lawns, storm drains, etc., and other landscaped facilities shall be borne by the Contractor unless otherwise noted.
- D. The following is a general outline of steps for the Directional Bore operation:
 - 1. Clear the right-of-way and temporary workspace as shown on the drawings. Contractor to install and maintain all soil erosion and sediment control devices, until project completion with approved permanent site stabilization.
 - 2. Lay out the pipe crossing alignment using a qualified land survey team to confirm accurate horizontal distances, either physically measured or shot by Electric Distance Measurement. Entry and exit points shall be located and marked with survey hubs or markers. Payment for survey mark-out shall be included in the price bid under horizontal directional drilling.
 - 3. Haul, string, and assemble restrained pipe. Joint air test the section prior to installation and hydrostatically test the assembled pipeline section, unless otherwise approved by Engineer. If sufficient linear footage of lay down area for the pipe string is not available, the finished pipeline may be assembled in no more than two sections, with each section joint air tested separately and hydrostatically tested when fully assembled as one piece. The Contractor will be responsible for ensuring that the drill rig has adequate pullback capacity to overcome the increased frictional resistance resulting from the stoppage of pipe pullback to perform the final weld or fusion of pipe sections. Provide adequate site security and shall be responsible for the integrity of the pipe until after the pullback, final test of the pipeline, and acceptance of the work by the Owner.

All assembled pipe sections shall be securely plugged at the end of each workday. The pipe interior is to be protected at all times against dirt, dust, drilling mud, pipe cuttings, debris, animal access, and other sources of contamination.

4. Provide adequate support rollers for the pipeline during pullback of the pipe string into the pre-drilled hole. The rollers and cradles shall be of a type that will prevent damage to the pipe and will be of sufficient number, as recommended by pipe manufacturer, to prevent over stressing due to sag bends during the pullback procedure. The pipe shall be supported at all times, including pullback, to maintain a free stress arc which limits pipe bending and internal hoop stresses to within manufacturer's limits.

Pipe which is not properly protected and supported and shows indications of excessive stressing, gouges, cuts, abrasions or other damage which may affect the operational performance intended for the pipe, as recommended by pipe manufacturer, shall be removed from the site and replaced at no additional cost as directed by the Owner or Engineer.

5. Mobilize the drilling equipment, erect the rig, drill a pilot hole, enlarge the hole as necessary to a minimum diameter of 1.5 times the nominal diameter of the pipe, and pullback the prefabricated pipe string under the crossing.

Prior to beginning the Pilot Hole over reaming, furnish to the Engineer with an as-built plan and profile of the actual crossing to confirm the installation is in compliance with the Contract Documents. Pilot hole alignment shall be accepted by Owner in writing prior to reaming and pipe installation.

The Contractor shall be responsible for selecting the reaming process to be utilized, whether forward and/or back reaming will be undertaken, and the number of reaming passes to be made.

- 6. Supply portable mud tanks or construct temporary mud pits to contain excess drill fluids during construction and slurry material displaced by the pipe during installation. Mud pits are to be protected at all times against unauthorized access and be stabilized at all times against surface water runoff and containment berm failure. Pump, haul and dispose of any drill cuttings and excess drill fluids to a receiving site permitted to accept the spoils, all in a manner consistent with the local and state regulations at no additional cost to the Owner.
- 7. Pull back the bore pipe in one continuous section and contractor using a swivel to minimize the rotation of the product pipe during pullback. Swivel shall utilize lubricated internal bearings which are fully protected from

external contamination and over lubrication. Demonstrate the swivel operation prior to pullback to the Engineer prior to the operation.

- 8. Use potable water and disinfect all piping and hoses used for water addition to the carrier pipe to counter the pipe flotation during pullback.
- 9. During pullback, maintain records for submission to Owner indicating job, date, time, constant pipe footage progress, mud flow rates, pulling forces required and torque readings. Document the pull head location for each length of drill stem pipe for as build records.
- 10. Unless not permitted by the right of way owner, inject low strength cement slurry into the bore hole for approximately 50 feet at each end of the drilled pipeline. Where cement slurry cannot be used, provide restraint at either end of the pipeline outside the bore to hold the pipe in place. The type of restraint shall be submitted to the Engineer in advance of the work and must be approved by the Engineer prior to the start of construction.
- 11. Owner and Engineer shall have access at all times to any measuring or gauging devices used for the horizontal drill as well as any drilling logs maintained by the Contractor.
- 12. In the event that the Contractor must abandon the drill hole before completion of the crossing, the Contractor will seal the borehole with neat cement grout starting at the low point or end of the drill hole and redrill the crossing at no extra cost to Owner.

3.05 PRESSURE TESTING AND LEAKAGE

Prior to pullback, perform an allowable leakage test on the full length of pipe after all sections have been welded or fused in accordance with ANSI/AWWA C600, latest revision and as described in Specification Section 15030. A hydrostatic pressure test shall also be performed on the installed pipe in accordance with ANSI/AWWA C600, latest revision and as described in Specification Section 15030.

3.06 CONNECTION TO ADJOINING PIPE

- A. Install flange connections from the directionally drilled pipe to adjacent pipe installed by open cut with support by backfill material as per Specification Section 02210. Flange bolts shall be carefully tightened in increments, with a final torque value not exceeding the manufacturer's recommendations. Tightening torque increments shall not exceed 15 foot-pounds.
- B. Polyethylene and flange gasket will undergo some compression set. Therefore, the flange bolts shall be retightened one hour after the initial assembly, and a second time at least four hours after the second tightening.

3.07 DISINFECTION

- A. The carrier pipe shall be disinfected as described in Specification Section 15020 or as otherwise approved in advance by the Engineer.
- B. The carrier pipe can be filled with potable water, pressure tested and disinfected prior to insertion. Provide Engineer with full work plan to employ this alternative.

3.08 AS-BUILT RECORDS:

The MGS pullback data shall be recorded every pilot hole drill stem length during the actual crossing operation. The Contractor shall furnish "as-built" plan and profile drawings, on the same horizontal and vertical control datum shown on the contract documents, based on these recordings showing the actual location horizontally and vertically of the installation, and all utility facilities found during the installation.

PART 4: EXECUTION

4.01 PERSONNEL REQUIREMENTS

- A. Responsible representatives of the Contractor and Drilling Subcontractor(s) shall be present at all times during directional drilling operations. A responsible representative as specified herein is defined as a person experienced in the type of Work being performed and who has the authority to represent the Contractor in a routine decision-making capacity concerning the manner and method of carrying out the Work.
- B. The Contractor and Drilling Subcontractor(s) shall have sufficient number of competent workers on the project at all times to ensure the utility placement is made in a timely, satisfactory manner. Adequate personnel for carrying out all phases of the directional drilling operation (where applicable: tunneling system operators, operator for removing spoil material, and laborers as necessary for various related tasks) must be on the job site at the beginning of Work. A competent and experienced supervisor representing the Contractor or Subcontractor that is thoroughly familiar with the equipment and type of Work to be performed, must be in direct charge and control of the operation at all times. In all cases, the supervisor must be continually present at the project site during the directional drilling operation.

4.02 WORK PLAN

- A. Prior to beginning Work, submit a Work plan to utilities detailing the procedure and schedule to be used to execute the project. The Work plan should include the following:
 - 1. A description of all equipment to be used;
 - 2. Down-hole tools;
 - 3. A list of personnel and their qualifications and experience;
 - 4. List of Subcontractors;
 - 5. A schedule Work activity;

- 6. A safety plan, traffic control plan (if applicable);
- 7. An environmental protection plan and;
- 8. Contingency Plans for possible problems.
- B. Work plan should be comprehensive, realistic and based on actual working conditions for this particular project. Plan should document the requirements to complete the project.
- C. Equipment
 - 1. Submit specifications on directional drilling equipment to be used to ensure that the equipment will be adequate to complete the project. Equipment shall include but not be limited to the following:
 - a. Drilling rig;
 - b. Mud system;
 - c. Mud motors (if applicable);
 - d. Down-hole tools;
 - e. Guidance system and;
 - f. Rig safety systems.
 - 2. Submit calibration records for guidance equipment for any drilling fluid additives that will or might be use.

4.03 COORDINATION OF THE WORK

- A. Notify Utilities at least three days in advance of starting Work. In addition, the actual crossing operation shall not begin until Utilities is present at the project site and agrees that proper preparations for the crossing have been made. Utilities' approval for beginning the crossing shall in no way relieve the Contractor from the ultimate responsibility for the completion of the Work.
- B. Coordinate with utilities to select a mutually convenient time for the crossing operation to begin in order to avoid schedule conflicts.

4.04 PROCEDURE

A. The installation of appropriate safety and warning devices in accordance with the "FDOT Manual on Traffic Control and Safe Practices" shall be completed prior to beginning Work.

4.05 INSTALLATION

A. Erosion and sedimentation control measures and on-site containers shall be installed to prevent drilling mud from spilling out of entry and/or exit pits. Drilling mud will be disposed of off-site in accordance with local, state and federal requirements and/or permit conditions. No other chemicals or polymer surfactant shall be used in the drilling fluid without written consent of Utilities and after a determination is made that the chemicals to be added are not harmful or corrosive to the facility and are environmentally safe.

- B. Pilot Hole: Pilot hole shall be drilled on bore path with no deviations greater than two percent of depth over a length of 100 feet. In the event that pilot does deviate from bore path more than two percent of depth in 100 feet, the Contractor will notify Engineer. The Engineer may require the Contractor to pull-back and re-drill from the location along bore path before the deviation.
- C. Reaming: Upon successful completion of pilot hole, ream borehole the to a minimum of 25 percent greater than outside diameter of pipe using the appropriate tools. Do not attempt to ream at one time more than the drilling equipment and mud system are designed to safely handle.
- D. Pullback: After successfully reaming borehole to the required diameter, put the pipe through the borehole. In front of the pipe will be a swivel and barrel reamer to compact bore hole walls. Once pullback operations have commenced, operations must continue without interruption until pipe is completely pulled into borehole. During pullback operations, do not apply more than the maximum safe pipe pull pressure at any time. A break away head rated at the maximum safe pull pressure shall be utilized.
- E. As-built variance from the designed bore path shall not exceed plus or minus one foot in the vertical plane and plus or minus two feet in the horizontal plane. Submit any proposed deviations from the design bore path with Shop Drawings.
- F. The pipe entry area shall be graded to provide support for the pipe to allow free movement into the borehole. The pipe shall be guided in the borehole to avoid deformation of, or damage to, the pipe.
- G. If unexpected subsurface conditions are encountered during the bore, the procedure shall be stopped. The installation shall not continue until the Owner and Engineer have been consulted.
- H. The pipe shall be pulled back through the borehole using the wet insertion construction technique. The pipe shall be installed full of water.
- I. The pipe shall be installed in a manner that does not cause upheaval, settlement, cracking, movement or distortion of surface features.
- J. A boring log shall be kept with horizontal and vertical location every 25 feet.

4.06 FIELD TESTING

- A. PVC Pipe: Perform hydrostatic testing for leakage following installation in accordance with the applicable test sections.
- B. HDPE Pipe: After installation the pipe shall be tested in accordance with the Manual with the following modifications:
 - 1. Test Duration: The total test time including initial pressurization, initial expansion and time at test pressure, must not exceed eight hours. If the test is not completed due to leakage, equipment failure, etc., the test

section shall be depressurized and allowed to "relax" for a minimum of eight hours before it is brought back up to test pressure. The test procedure consists of initial expansion phase and leakage test phase.

- 2. Initial Expansion Phase: During the initial expansion phase, the test section is pressured to the test pressure and enough make-up liquid is added each hour for three hours to return to test pressure.
- 3. Leakage Test Phase: The leakage test phase follows immediately and shall be either two or three hours in duration. At the end of the time test, the test section shall be returned to test pressure by adding a measured amount of liquid. The amount of make-up liquid added shall not exceed the values provided in Table 3.15-6 plus allowable leakage.

Test Duration	Pipe Diameter (inches)							
(hours)	2	4	6	8	12	16	20	24
2	0.11	0.25	0.60	1.00	2.30	3.30	5.50	8.90
3	0.19	0.40	0.90	1.50	3.40	5.50	8.00	13.00
Allowance/100 feet of Pipeline (gallons)								

Table 3115-6. Allowance for Make-up Water Under Test Pressure*

*Applies to test period and not to initial expansion phase

- C. Pressure Testing: The test pressure for the pipe shall be 150 psi for water and reclaimed water and 100 psi for wastewater.
- D. Mandrel Testing: Perform mandrel testing through the entire length of the installed pipe. The mandrel size shall be 90 percent of the inside diameter of the pipe.

END OF SECTION

SECTION 02458

LARGE SCALE HORIZONTAL DIRECTIONAL DRILLING (HDD) (Projects Greater Than 250 Feet or Pipe Size Greater Than 12 Inches)

PART 1: GENERAL

1.01 SCOPE

A. Furnish all labor, materials, tools and equipment as necessary to construct a pipeline crossing by the horizontal directional drilling method. Furnish all labor, equipment, materials and supplies and perform all work necessary to provide Owner with a complete, finished water main crossing. The finished work includes proper installation testing, restoration of underground utilities and environmental protection and restoration.

1.02 RELATED SECTIONS

Section 01300 – Submittals Section 02200 – Excavation, Backfilling and Compaction Section 15000 – Piping - General Provisions Section 15020 – Disinfecting Pipelines

1.03 QUALITY ASSURANCE

- A. The HDD equipment operator(s) shall be trained to operate the specific HDD equipment for the Owner's project with at least 3 years experience in directional drilling obtained within the last five years. All pipe and appurtenances of similar type and material shall be furnished by a single manufacturer.
- B. Perform HDD operations under the constant direction of a drilling supervisor who shall remain on site and be in responsible charge throughout the drilling operation. The Contractor's supervisor shall have supervised directional drilling of a minimum of 5,000 linear feet of pipe of a similar or greater diameter, of similar material, over similar lengths, and with similar subsurface conditions.
- C. The requirements set forth in this Specification specify a wide range of procedural precautions necessary to ensure that the basic, essential aspects of a proper Directional Bore installation are adequately controlled. Strict adherence shall be required under specifically covered conditions outlined in this Specification.
- D. Perform the work in general conformance with ASTM F1962-05, current revision, "Standard Guide for Use of Maxi-Horizontal Directional Drilling for Placement of Polyethylene Pipe of Conduit under Obstacles, Including River Crossings."
- E. Adhere to the specifications; any changes must be expressly approved by the Engineer. Approval of any aspect of any Directional Bore operation covered by this Specification shall in no way relieve the Contractor of its ultimate

responsibility for the satisfactory completion of the work authorized under the Contract.

1.04 PROFILES AND TOPOGRAPHY

A. Contours, topography and profiles of the ground as may be shown on the Contract Drawings are believed to be reasonably correct but are not guaranteed to be absolute and are presented only as an approximation. It is the Contractor's responsibility to verify all elevations required to successfully complete the crossing.

1.05 SUBMITTALS

- A. Prior to beginning work, submit to the Engineer copies of a report of schedules, calculations, procedures and any supplemental subsurface soil condition investigations performed along the path of the proposed crossing. Number of copies of the report shall be as specified in Section 01300. The report will summarize the subsurface conditions that are known to the Contractor and that his proposed crossing procedure is based upon factual, best available information. If the subsurface conditions are known to the Contractor by previous work or geotechnical studies done in the immediate area, the information shall be recorded in the report along with any additional geotechnical studies performed by the Contractor. The report shall include the following:
 - 1. <u>Subsurface Information</u>
 - a. Record in the report subsurface conditions known to the Contractor by previous work or prior geotechnical studies performed in the immediate project area.
 - b. Boring information obtained by the Owner, if any, is listed in the Supplementary Conditions section of these Specifications.
 - c. Additional borings performed by the Contractor and analysis of soils along the path of the proposed crossing. The Contractor shall be responsible for obtaining and including in his bid price the cost of any additional borings along the pipe alignment which may be necessary to design the proposed directionally drilled crossing.

At a minimum any supplemental borings performed by the Contractor shall include standard classification of soils, standard penetration tests, split spoon sampling and sieve analysis. Test borings shall be performed to a minimum depth of ten (10) feet below the proposed pipe invert unless rock is encountered in which case test borings shall penetrate at least two feet into rock.

- 2. Drilling Equipment and Methods
 - a. Submit information on equipment and written procedure with working drawings describing in detail the proposed boring method and the entire operation to be used. This shall include, but not be limited to, entry and exit pits; settlement pit; size, capacity and arrangement of drilling and pulling equipment; layout of carrier pipe;

details and spacing of pipe rollers; type of current head; method of monitoring and controlling line and grade; method of detection of surface movement; and layout of any proposed construction staging areas.

- b. In addition, submit for approval nameplate data for the drilling equipment, mobile spoils removal unit, and Material Safety Data Sheets (MSDS) information for the drilling slurry compounds. This must be submitted and reviewed by the Engineer before work can proceed.
- 3. Piping

Submit shop drawings showing the pipe lengths, design details, joint details, etc. for the Engineer's review. Submittals shall include, but are not limited to, the following:

- a. All welding or fusion procedures to be used in fabrication of the different pipe materials and installation methods.
- b. Certified records for hydrostatic testing of all pipe materials to be used.
- c. An affidavit stating that all pipe materials furnished under this section have been manufactured in the United States of America and comply with all applicable provisions of referenced AWWA standards.
- 4. Proposed Alignment

Submit a graph in plan and profile plotting the pilot drilling hole alignment to the Engineer for review, including entry/exit angles and radius of curvature. After completion of the crossing, submit a final pipe alignment.

5. <u>Schedule</u>

Time schedule for completing the Directional Bore, including any delays due to anticipated soil conditions.

- 6. Calculations
 - a. Submit detailed design calculations for several representative loading conditions for the proposed crossing. If requested by the Engineer, submit calculations to support the design of any particular location of pipe anywhere along the length of the crossing at no additional cost to the Owner.
 - b. Design calculations shall be presented in a neat, readable format, with all figures, values and units included to facilitate ease of verification.
 - c. Calculations shall be submitted to demonstrate that the pipe thickness design is sufficient to meet all design criteria specified.

- d. Calculations shall address the following loading conditions:
 - Pre-installation: Hoop and longitudinal stress during hydrostatic test; spanning stress with pipe full of water and supported on installation rollers, and maximum roller / support spacing.
 - ii) Installation/Post-Installation: Longitudinal stress from pulling force; longitudinal curvature stress at point of entry and in final position; external pressure from drilling fluid, overburden, and loads from the obstacle being crossed.
 - iii) Post-Installation/In-Service: Hoop and longitudinal stress during hydrostatic test; internal working and surge pressure; buckling with internal vacuum.
- e. Perform and submit to the Engineer fluids pressure versus overburden strength calculations. These calculations shall be performed to determine minimum acceptable cover requirements and prevent drilling fluids breakout to the ground surface.
- f. All calculations shall bear the seal of a Registered Professional Engineer. Licensure in the State that the work is performed is preferred.

B. <u>Approval</u>

1. No work shall commence without approval by the Engineer. Details and design calculations shall be submitted and approved well in advance of the drilling operation to prevent delays in work. All final layout work, including grades, shall be the Contractor's responsibility.

1.06 JOB CONDITIONS

- A. <u>Any</u> nighttime work is <u>strictly regulated</u> and will be allowed only with prior approval granted by the Owner <u>subject to</u> regulatory agencies having jurisdiction. All crossing operations shall be accomplished during daylight hours, unless approved by the Engineer. Crossing work shall not begin after the hour pre-established as the latest starting time that will allow completion during daylight hours, unless approved by the engineer. The Contractor shall provide a Work Plan submittal indicating its proposed hours of operation and length of work week. All work plans shall be subject to compliance with all applicable regulatory requirements for construction activities and any off-site impacts.
- B. When hazards of night time work are carefully considered and determined to be insignificant, night time work may be allowed only to complete a properly planned crossing, and only if in the opinion of the Engineer the delay was caused by reasonably unavoidable circumstances, and that such night time work is necessary to avoid placing an undue economic hardship on the Contractor.

- C. In emergency situations, or where delay would increase the likelihood of a failure, nighttime work may be allowed to complete a delayed crossing.
- D. All operations shall continue on a 24-hour per day basis during pipe pull back.

1.07 COORDINATION OF WORK

A. Coordinate connections to existing pipelines that require shutdown of Owner facilities. Owner will designate the time for these connections that could involve work during evenings, nights, Saturdays, Sundays, or holidays. Method of connection and designated times are to cause the least amount of disruption to Owner's water service to its customers. The cost for connections is to be included in the contract price. No contract price adjustment will be allowed for overtime, premium time, or other related costs.

1.08 USE OF EXISTING WATER SYSTEMS

- A. All use of existing water systems during construction by the Contractor shall be with the approval and direction of the system Owner and its representatives. The Contractor shall be responsible for all permits, fees, temporary piping, temporary meter rental/provisions, temporary backflow preventer rental/provision and other water utility requirements for supplying water during construction. The Contractor shall use the existing water system only at locations, times and conditions as set forth by the system owner or its representatives.
- B. If water is not readily available at the site or the Owner cannot provide the volume of flow required by the Contractor, provide potable water as needed from an off-site location at no additional cost to the Owner.

PART 2: PRODUCTS

2.01 PIPE

Unless otherwise specified in the Contract Documents, pipe installed by horizontal directional drilling shall either be high density polyethylene pipe (HDPE), steel pipe, or ductile iron pipe specifically designed for directional drilling. Unless otherwise specified in the Contract Documents, the water main pipe (carrier pipe) shall be installed without a casing pipe.

A. POLYETHYLENE PIPE

- 1. High Density Polyethylene (HDPE) Pipe, AWWA C906 compliant, NSF 61 Standard Listed, and furnished in fifty (50) foot lengths.
- 2. Polyethylene pipe shall be furnished with an outside diameter conforming to ductile iron pipe sizes. Minimum thickness of HDPE pipe shall be determined by the contractor's calculations but shall not be considering inservice loading shall not be less than DR 11 when measured in accordance with ASTM D2122.

- 3. All polyethylene pipe and fittings shall be made of a high-density polyethylene pipe compound with extra high molecular weight that meets the requirements for Type III, Grade P34 Polyethylene material as defined in ASTM D1248, latest revision.
- 4. Pipes shall be jointed to one another and to polyethylene fittings by thermal butt-fusion or by socket fusion in accordance with ASTM D3261.
- 5. Joining of pipe sections shall be performed in accordance with the procedures recommended by the pipe manufacturer. Joints between pipe sections shall be smooth on the inside and internal projection beads shall not be greater than 3/16-inch.
- 6. The tensile strength at yield of the butt-fusion joints shall not be less than the pipe. A specimen of pipe cut across the butt-fusion joint shall be tested in accordance with ASTM D638.
- 7. Polyethylene pipe shall be joined to ductile iron pipe by the use of flange adapters and back-up rings. Flange adapters shall be butt fused to the polyethylene carrier pipe. The face of the flange adapter shall have a serrated sealing face to assist in holding the flange gasket in place. Flange gaskets shall be full-faced neoprene. Back-up rings shall be Class "D" steel ring flanges in accordance with AWWA C207. Flange bolts must span the entire width of the flange joint and provide sufficient thread length to fully engage the nut.

B. STEEL PIPE

- 1. Steel pipe shall meet the requirements of AWWA C200 and Specification Section 15110.
- 2. Steel pipe sections shall be connected by welding. All welding shall conform to AWWA C206, latest revision. Pipe shall be either spiral seam or longitudinally rolled pipe.
- All steel pipe shall receive an interior and exterior factory coating of fusionbonded epoxy, 20-mil minimum thickness. Material and application requirements shall be as specified in AWWA C213, latest edition, "Standard for Fusion - Bonded Epoxy Coating for the Interior and Exterior of Steel Water Pipelines".
- 4. The interior and exterior of field-welded joints shall receive a 25-mil minimum thickness coating of fusion-bonded epoxy, applied in accordance with AWWA C213.
- 5. Minimum thickness of steel pipe shall be determined by the contractor's calculations but shall not be less than a diameter to thickness ratio of 180.

C. DUCTILE IRON PIPE

- 1. Utilize ductile iron pipe equipped with low profile flexible restrained joints such as Flex Ring or TR Flex. Gripping push on joint gaskets, or restrained joint gaskets are not permitted.
- 2. All ductile iron pipe shall be installed per DIPRA's Horizontal Directional Drilling with Ductile Iron Pipe Handbook to include strict adherence to maximum joint deflection allowances

D. THICKNESS DESIGN

The following design criteria shall be used in calculating pipe thickness for HDPE, steel, or ductile iron pipe:

Working Pressure Test Pressure Surge Pressure Dead Load	PSI as provided by Owner. PSI as provided by Owner. Working pressure + 100 psi Earth cover as shown on Drawings, but not less than 15 feet.
Buckling Design	Considering dead load, internal vacuum, H-20 Wheel Loading and a hydrostatic load over top of pipe to grade.
Max. Allowable Horizontal Deflection	3%
Radius of Curvature	90% of Actual Design Radius
Downhole Friction Factor Factor of Safety for	1.0
Drilling Fluid Density	1.5

The stresses in the pipe shall be calculated for the pre-installation, installation, and post installation loading conditions specified in Part 1 of this Specification Section. Thickness shall be selected so that stresses do not exceed the following under any of the loading conditions.

- 1. All conditions except internal surge pressure: 50% of minimum yield point
- 2. Internal surge pressure condition: 75% of minimum yield point

The contractor shall increase the minimum "in-service" thickness as necessary to support the expected stresses and loadings which are expected to be encountered during the installation of the HDD pipeline. The final selected thickness shall be supported by calculations as required herein. No additional cost shall be considered by the Owner for pipe thickness greater than the specified minimum "in-service" thickness.

E. DEVIATIONS

Should the Contractor choose to submit a bid using material that does not meet all the requirements of these specifications, include a description of the deviation with data showing the magnitude of the deviation. Acceptance of such deviations to these specifications shall be subject to the review and approval of the Owner before a contract can be awarded.

F. INSPECTION OF PIPE

All pipe and fittings used in the work may be factory inspected by a recognized agency engaged by the Owner. Inform the Owner and the inspection agency of the name and address of the manufacturing plant or other sources of materials to be used in the work and shall coordinate with the manufacturer to assure that the inspection agency has access at the manufacturer's plant and adequate assistance and notice so that each item may be examined. All reports will be made to the Owner and the cost of the services of the inspection agency will be borne by the Owner. Such inspection by the Owner shall not relieve the Contractor of his responsibility to furnish materials in accordance with the applicable standards.

2.02 EQUIPMENT

- A. General: All equipment for the Directional Bore shall have the capacity, stability, and necessary safety features required to fully comply with the specifications and requirements of this section without showing evidence of undue stress or failure. It shall be the responsibility of the Contractor to assure that the equipment to be used in the Directional Bore is in sound operating condition. Backup equipment shall be required in the event of an equipment breakdown and where the condition of the equipment to be used indicates that routine component replacement or repair will likely be necessary during the Directional Bore.
- B. Directional Drilling System: The directional drilling system shall consist of over the road transportable field power unit, mud-mixing and recycling unit, a trailer or carriage-mounted drill unit, and all other support accessory vehicles and equipment. All system components shall be in sound operating condition with no broken welds, excessively worn parts, badly bent, or otherwise misaligned components. All drill pipe, reamers, pull back heads, swivels, drill heads and collars, pipe cradles, pipe rollers, ropes, cables, clamps, and other nonmechanical but essential items shall be in sound condition and replaced immediately when need is apparent. The equipment must be capable of drilling the specified length in a single bore.
 - 1. <u>Mud-Mixing and Recycle Units</u>: The mud-mixing and recycle unit shall be a self-contained system designed to provide a supply of high-pressure Bentonite based cutting fluid to the drill unit. It shall contain a fluid storage tank and a complete Bentonite and drilling fluid additive(s) mixing system. The cutting fluid is to be mixed on site. The cutting fluid shall be formulated for this specific project and anticipated conditions. It shall permit changes to be made to the Bentonite and drilling fluid additive(s) concentrations during drilling in response to changing soil conditions. The field power unit shall contain the power-taken off-driven high pressure

cutting fluid pumping system. The recycle units shall be of a capacity to minimize the production of new cutting fluid and maximize the reuse and recirculation of original cutting fluid produced.

2. <u>Directional Drill System</u>: A carriage-mounted version of the drill system shall include a thrust frame. Both the trailer-mounted and carriage-mounted drill system shall be designed to rotate and push 10-foot (3-meter) minimum hollow drill sections into the tunnel being created by the boring head. The drill sections shall be made of a high strength S-grade steel that permits them to bend to a 30-foot (9-meter) radius without yielding. Drill end fittings shall permit rapid makeup of the drill sections while meeting the torque, pressure and lineal load requirements of the system. The boring head itself shall be capable of housing a probe used by the Magnetic Guidance System (MGS) to determine tool depth and location from surface and to orient the head for steering. The MGS shall have a minimum accuracy of ± two (2) percent of the vertical depth.

The drilling equipment must be fitted with a permanent alarm system capable of detecting an electric current. The system will have an audible alarm to warn the operator when the drill head nears electrified cables. The drilling equipment shall be grounded, protected, and operated in accordance with manufacturer's requirements for electric strike safety.

The control console shall contain a calibrated display of inclination, azimuth, tool face location, mud pump rates, and torque pressures. The downhole steering system accuracy shall be plus or minus one percent (\pm 1.0%) of the horizontal bore length such that the difference between actual depth and machine calculated depth is not more than 1 foot per hundred feet.

- 3. <u>Restrictions</u>: Other devices or utility placement systems for providing horizontal thrust other than those previously defined in the preceding sections shall not be used unless approved by the Engineer prior to commencement of the work. The proposed device or system will be evaluated prior to approval or rejection on its potential ability to complete the pipe placement satisfactorily without undue stoppage and to maintain line and grade within the tolerances prescribed by the particular condition of the project. Water sluicing methods, jetting with compressed air, or boring or tunneling devices with vibrating type heads that do not provide positive control of the line and grade shall not be allowed.
- C. Spoils Equipment: The cutting fluid removal system shall include a self-contained vacuum truck which has sufficient vacuum and tank capacity to remove excess cutting fluid mixture and cuttings from the project site as required or directed by the Engineer. Spoils are not to be discharged into sewers or storm drains.

The Contractor will contain all drilling and pipe lubricating mud by taking special measures to prevent run-off into adjacent properties and/or waterways. All surplus drilling and pipe lubricating mud will be removed from the site and properly disposed of by the Contractor. The Contractor will also be responsible for all required erosion control measures.

D. Magnetic Guidance System: A Magnetic Guidance System (MGS) probe and location of the drill head during the drilling operation. The tracker shall be capable of tracking at all depths up to one hundred feet and in any soil condition, including hard rock. It shall enable the driller to guide the drill head by providing immediate information on the tool face, azimuth (horizontal direction), and inclination (vertical direction). The tracker shall be accurate to ±2% of the vertical depth of the borehole at sensing position at depths up to one hundred feet. Ferrous materials shall not influence or affect the MGS readings or accuracy.

Components: The Contractor shall supply all components and materials to install, operate, and maintain the MGS. This shall include, but not be limited to the following:

- 1. MGS Probe and Interface
- 2. Computer, Printer, and Software
- 3. DC Power Source, Current Control Box, and Coil/Tracking Wire.

The Magnetic Guidance System (MGS) shall be a Tensor TruTracker MGS, or other licensed and industry approved wire guidance system. The Engineer shall be advised of the unit to be used and is subject to his approval. Set up and operate the MGS using personnel experienced with this system. A Walk-over" tracking systems shall not be used, except as approved by the Engineer. Contractor shall provide Engineer with current calibration certification of MGS in accordance with manufacturer's specifications.

- E. If equipment breakdown or other unforeseen stoppages occur and forward motion of the directional cutting head is halted at any time other than for reasons planned in advance (addition of drill stems, etc.), the boring path shall be filled with a proper Bentonite solution immediately, or as directed by the Engineer.
- F. The boring tool shall have steering capability and have an electronic tool detection system. The position of the tool during operation shall be capable of being determined accurately, horizontally within 1% of the horizontal distance of the borehole and vertically within 2% of the vertical depths of the borehole. The boring tool shall have a nominal steering radius of 9 meters (30 feet).

2.03 DRILLING FLUIDS:

A. A mixture of Bentonite drilling clay, project specific cutting fluid additives, and potable water is to be used as the cutting fluid (MUD) and over ream hole filler for the Directional Bore. The drilling fluid mixture used shall have the following minimum viscosities as measured by a March Funnel:

Rock Clay	60 sec.
Hard Clay	40 sec.
Soft Clay	45 sec.
Sandy Clay	90 sec.
Stable Sand	120 sec.
Loose Sand	150 sec.
Wet Sand	150 sec.

These viscosities may be varied to best fit the soil conditions encountered as recommended by the drilling mud and fluid additive manufacturer, and as approved by the Engineer.

- B. Where sandy or granular materials are encountered, a cement slurry or polymer supplement shall be considered for added strength and stability of the bore and over ream hole.
- C. No chemicals or polymer surfactant shall be used in the drilling fluid without written consent of the Engineer, and after a determination is made that the chemicals to be added are not harmful or corrosive to the facility and are environmentally safe. Clay must be totally inert and contain no risk to the environment.
- D. Provide Owner, Engineer and have on site at all times the Material Safety Data Sheets (MSDS) for all drilling compounds and chemicals.

2.04 TRACER WIRE

- A. When HDPE pipe is used, tracer or location wire shall be a direct burial #12 AWG Solid (0.0808 inch diameter), steel core hard drawn extra high strength horizontal directional drill tracer wire, 1150# average tensile break load, 45 mil. High molecular weight-high density blue polyethylene jacket complying with ASTM D1248, 30-volt rating. The wire shall be contiguous except at test stations, valve boxes, and where splicing is required. All splices shall be encased with a 3M-Gel Pack model No. 054007-09053. Wire insulation shall be highly resistant to alkalis, acid and other destructive agents found in soil. Location Wire shall be from Copperhead Industries, LLC, part number 1230B-HS or approved equal
- B. Tracer wire shall be installed simultaneously with pullback of the HDPE pipe. Wire shall either be wrapped around the pipe or taped to the pipe at 10-foot minimum intervals before installation.

PART 3: EXECUTION

3.01 SITE DISTURBANCE AND SOIL EROSION

- A. Sediment barriers shall be constructed as shown on the Drawings or where directed by the Engineer. All soil erosion and sediment control work shall be done in accordance with the Standards for Soil Erosion and Sediment Control for the location where the work is performed. Contractor shall maintain sediment barriers until the project is deemed complete.
- B. The Contractor shall be responsible for the preservation of all existing trees, plants, and other vegetation that are to remain within or adjacent to the construction site and shall also be responsible for protecting existing concrete curb, fence, utilities, and other structures that are located within or adjacent to the construction site.

C. The Contractor assumes all liability for environmental damage and cleanup due to inadvertent discharges of slurry or other causes. Slurry materials shall be selected based on the soil conditions encountered to minimize the risk of mud returns.

3.02 PERSONNEL REQUIREMENTS

- A. Provide a competent and experienced supervisor representing the Drilling Contractor who must be present at all times during actual operations. A responsible representative, who is thoroughly familiar with the equipment and type work to be performed, must be in direct charge and control of the operation at all times. In all cases the supervisor must be continually present at the job site during the actual Directional Pilot Hole, over reaming and pullback operations.
- B. Have a sufficient number of competent workers on the job at all times to ensure the Directional Bore is made in a timely and satisfactory manner. Adequate personnel for carrying out all phases of the actual Directional Bore operation must be on the job site at the beginning of work.
- C. If HDPE is specified for the carrier pipe, HDPE pipe thermal butt fusion welding is to be completed by a welder certified by the manufacturer of the pipe or pipe welding equipment, in accordance with the Plastic Pipe Institute "Handbook of Polyethylene Pipe," Polyethylene Joining Procedures, and 49 CFR 192, Subpart F, latest edition.
- D. If steel pipe is specified for the carrier or casing pipe, welding shall be performed by certified welders. The Contractor shall be responsible for the qualification of welders with qualification testing conducted by an independent testing agency in accordance with American Welding Society D1.1 requirements. Results of qualification testing shall be submitted to the Engineer for approval. Results of previous qualification tests performed within six months from the date of pipe installation will be acceptable. Results from qualification tests performed prior to six months from the date of pipe installation will not be acceptable. All costs associated with qualification testing shall be included in the unit prices bid.
- E. The Engineer and Owner must be notified 48 hours in advance of starting each phase of the work. The Directional Bore shall not begin until the Engineer is present at the job site and agrees that proper preparations for the operation have been made. The Engineer's approval for beginning the installation shall in no way relieve the Contractor of the ultimate responsibility for the satisfactory completion of the work as authorized under the Contract. It shall be the responsibility of Owner to provide inspection personnel at such times as appropriate without causing undue hardship by reason of delay to the Contractor.
- F. If the Contractor fails to begin the Directional Bore at the agreed time, the Owner will establish the next mutually convenient time to begin. To avoid undue hardship of either party, reasonable and mutual cooperation should be exercised where starting times are concerned. If one party fails to meet the agreed schedule, the other party is expected to consider a delayed start if the installation

cannot be completed during daylight hours.

3.03 ALIGNMENT AND GRADE

- A. Determine and physically locate the depth, location, and size of all existing underground facilities in the vicinity of the proposed crossings and provide the Engineer with a comprehensive report of these facilities before starting any construction. The Contractor shall be held completely and solely responsible for any damages incurred. The kinds, locations and sizes of the existing underground utilities which may be shown on the Contract Drawings are intended only as a guide to the Contractor and are not guaranteed to be even approximately correct. Notify the owners of all existing utilities along the route and in the vicinity of the crossing prior to the construction to include all test borings and excavations.
- B. If utilities of unknown depth or other obstructions require grade or alignment deviations from the Plans, the grade and/or alignment may be adjusted with Engineer's approval. All adjustments shall permit gradual bends of the pipe to the original alignment beyond the directional bore section. At unusual site conditions, the Contractor may request a review of site conditions by the Engineer for additional adjustment, and such determination shall be final. An adjustment in alignment, position, or elevation approved by Engineer shall not be cause for an adjustment of costs.
- C. Pipe entry and exit points are to be allowed no more than five (5) feet of deviation from the staked centerline. The entry point may be moved up to twenty-five (25) feet further from the original entry point only with Engineer's approval. Exit point lengths greater than twenty-five (25) feet from the original point require Engineer's approval. Entry and exit points normally will not be allowed closer to the banks of a waterway being crossed. Any installation that deviates from the plan may be rejected and any rejected installation shall be reconstructed at the Contractor's expense.
- D. The vertical profile as shown on the drawings is the minimum depth to which the pipeline shall be installed. Contractor may, at his option and with the permission of Owner, elect to install the pipe at a greater depth than shown on the drawings, at no additional cost to the Owner.

3.04 INSTALLATION

A. The Contractor shall be responsible for providing a Maintenance of Traffic Plan to the Engineer and local traffic law enforcement agency for review. The Maintenance of Traffic Plan shall show the location of all barricades, signs, devices and alternate routes for local traffic and pedestrian safety. Erection of the appropriate safety and warning devices in accordance with the USDOT "Manual of Uniform Traffic Control Devices" (MUTCD) shall be completed prior to beginning work and maintained until all construction is completed and the site restored.

- B. Specifically note in the Maintenance of Traffic Plan street intersections that are to remain open as required during the pipe pull-back operation, or traffic detours implemented. Install a temporary sleeve across the street intersections through which the pipe can be pulled or to construct a temporary bridge for the pipe over the intersections as required. No additional payment will be made for temporary structures required in order to permit access through street intersections or the implementation of traffic detours.
- C. The cost of restoring pavement, curb, sidewalk, driveways, lawns, storm drains, etc., and other landscaped facilities shall be borne by the Contractor unless otherwise noted.
- D. The following is a general outline of steps for the Directional Bore operation:
 - 1. Clear the right-of-way and temporary workspace as shown on the drawings. Contractor to install and maintain all soil erosion and sediment control devices, until project completion with approved permanent site stabilization.
 - 2. Lay out the pipe crossing alignment using a qualified land survey team to confirm accurate horizontal distances, either physically measured or shot by Electric Distance Measurement. Entry and exit points shall be located and marked with survey hubs or markers. Payment for survey mark-out shall be included in the price bid under horizontal directional drilling.
 - 3. Haul, string, and assemble restrained pipe. Joint air test the section prior to installation and hydrostatically test the assembled pipeline section, unless otherwise approved by Engineer. If sufficient linear footage of lay down area for the pipe string is not available, the finished pipeline may be assembled in no more than two sections, with each section joint air tested separately and hydrostatically tested when fully assembled as one piece. The Contractor will be responsible for ensuring that the drill rig has adequate pullback capacity to overcome the increased frictional resistance resulting from the stoppage of pipe pullback to perform the final weld or fusion of pipe sections. Provide adequate site security and shall be responsible for the integrity of the pipe until after the pullback, final test of the pipeline, and acceptance of the work by the Owner.

All assembled pipe sections shall be securely plugged at the end of each workday. The pipe interior is to be protected at all times against dirt, dust, drilling mud, pipe cuttings, debris, animal access, and other sources of contamination.

4. Provide adequate support rollers for the pipeline during pullback of the pipe string into the pre-drilled hole. The rollers and cradles shall be of a type that will prevent damage to the pipe and will be of sufficient number, as recommended by pipe manufacturer, to prevent over stressing due to

sag bends during the pullback procedure. The pipe shall be supported at all times, including pullback, to maintain a free stress arc which limits pipe bending and internal hoop stresses to within manufacturer's limits.

Pipe which is not properly protected and supported and shows indications of excessive stressing, gouges, cuts, abrasions or other damage which may affect the operational performance intended for the pipe, as recommended by pipe manufacturer, shall be removed from the site and replaced at no additional cost as directed by the Owner or Engineer.

5. Mobilize the drilling equipment, erect the rig, drill a pilot hole, enlarge the hole as necessary to a minimum diameter of 1.5 times the nominal diameter of the pipe, and pullback the prefabricated pipe string under the crossing.

Prior to beginning the Pilot Hole over reaming, furnish to the Engineer with an as-built plan and profile of the actual crossing to confirm the installation is in compliance with the Contract Documents. Pilot hole alignment shall be accepted by Owner in writing prior to reaming and pipe installation.

The Contractor shall be responsible for selecting the reaming process to be utilized, whether forward and/or back reaming will be undertaken, and the number of reaming passes to be made.

- 6. Supply portable mud tanks or construct temporary mud pits to contain excess drill fluids during construction and slurry material displaced by the pipe during installation. Mud pits are to be protected at all times against unauthorized access and be stabilized at all times against surface water runoff and containment berm failure. Pump, haul and dispose of any drill cuttings and excess drill fluids to a receiving site permitted to accept the spoils, all in a manner consistent with the local and state regulations at no additional cost to the Owner.
- 7. Pull back the bore pipe in one continuous section and contractor using a swivel to minimize the rotation of the product pipe during pullback. Swivel shall utilize lubricated internal bearings which are fully protected from external contamination and over lubrication. Demonstrate the swivel operation prior to pullback to the Engineer prior to the operation.
- 8. Use potable water and disinfect all piping and hoses used for water addition to the carrier pipe to counter the pipe flotation during pullback.
- 9. During pullback, maintain records for submission to Owner indicating job, date, time, constant pipe footage progress, mud flow rates, pulling forces required and torque readings. Document the pull head location for each length of drill stem pipe for as build records.

- 10. Unless not permitted by the right of way owner, inject a low strength cement slurry into the bore hole for approximately 50 feet at each end of the drilled pipeline. Where cement slurry cannot be used, provide restraint at either end of the pipeline outside the bore to hold the pipe in place. The type of restraint shall be submitted to the Engineer in advance of the work and must be approved by the Engineer prior to the start of construction.
- 11. Owner and Engineer shall have access at all times to any measuring or gauging devices used for the horizontal drill as well as any drilling logs maintained by the Contractor.
- 12. In the event that the Contractor must abandon the drill hole before completion of the crossing, the Contractor will seal the borehole with neat cement grout starting at the low point or end of the drill hole and re-drill the crossing at no extra cost to Owner.

3.05 PRESSURE TESTING AND LEAKAGE

A. Prior to pullback, perform an allowable leakage test on the full length of pipe after all sections have been welded or fused in accordance with ANSI/AWWA C600, latest revision and as described in Specification Section 15030. A hydrostatic pressure test shall also be performed on the installed pipe in accordance with ANSI/AWWA C600, latest revision and as described in Specification Section 15030.

3.06 CONNECTION TO ADJOINING PIPE

- A. Install flange connections from the directionally drilled pipe to adjacent pipe installed by open cut with support by backfill material as per Specification Section 2210. Flange bolts shall be carefully tightened in increments, with a final torque value not exceeding the manufacturer's recommendations. Tightening torque increments shall not exceed 15 foot-pounds.
- B. Polyethylene and flange gasket will undergo some compression set. Therefore, the flange bolts shall be retightened one hour after the initial assembly, and a second time at least four hours after the second tightening.

3.07 DISINFECTION

- A. The carrier pipe shall be disinfected as described in Specification Section 15020 or as otherwise approved in advance by the Engineer.
- B. The carrier pipe can be filled with potable water, pressure tested and disinfected prior to insertion. Provide Engineer with full work plan to employ this alternative.

3.08 AS-BUILT RECORDS

A. The MGS pullback data shall be recorded every pilot hole drill stem length during the actual crossing operation. The Contractor shall furnish "as-built" plan and profile drawings, on the same horizontal and vertical control datum shown on the contract documents, based on these recordings showing the actual location horizontally and vertically of the installation, and all utility facilities found during the installation.

END OF SECTION

SECTION 02540

EROSION AND SEDIMENTATION CONTROL

PART 1: GENERAL

1.01 SCOPE OF WORK

Work to be performed under this Specification Section refers to temporary and permanent vegetation covers, mulching, and baling at the construction site and all areas disturbed during construction, including borrow areas. In addition to the requirements of these Specifications, comply with all local Conservation District laws, rules and regulations and all other Federal, State, County and local requirements for erosion and sedimentation control.

1.02 STANDARDS

Comply with the highest erosion and sedimentation control standards, whether Conservation District, Federal, State or local. If in doubt as to the applicable standard, notify the Engineer and comply with the Engineer's directions concerning the prevailing jurisdiction.

PART 2: PRODUCTS

2.01 MATERIALS - GENERAL

All materials such as seeds, mulch, silt fencing and bales shall conform to the Specifications of the local Conservation District and all other applicable Federal, State, County and local requirements.

PART 3: EXECUTION

3.01 GENERAL

- A. Submit plan to comply with regulators and Engineer for approval using established best practices. Construct silt fences, diversion ditches with catch basins and drains as shown on the Plans prior to any other construction activity.
- B. Drain the settled water from the catch basins to the natural local drains. Clean the catch basins regularly. After final grading, seed and mulch the area per Specification Sections 1.02 and 2.01.
- C. Permanent vegetation cover, mulching, and baling shall be in accordance with the Conservation District specifications and all other applicable Federal, State and local requirements.

END OF SECTION

SECTION 02558

IDENTIFICATION/LOCATION GUIDE

PART 1: GENERAL

1.01 SCOPE

A. Furnish and install identification tape and location wire over the centerline of buried potable water mains, hydrant branches, and trenched services as indicated in this specification or noted in the drawings.

PART 2: PRODUCTS

2.01 IDENTIFICATION TAPE

A. Identification Tape for Pipe

Identification tape shall be manufactured of polyethylene with a minimum thickness of 4 millimeters and shall have a 1-millimeter thick metallic foil core. The tape shall be highly resistant to alkalis, acid and other destructive agents found in soil. Tape width shall be a minimum of 3 inches and a maximum of 6 inches and shall have the background color specified below, imprinted with black letters. Imprint shall be as specified below and shall repeat itself a minimum of once every 2 feet for entire length of the tape.

B. Tape background colors and imprints shall be as follows:

Imprint "CAUTION - WATER LINE BURIED BELOW" Background Color Blue

C. Identification tape shall be "Terra Tape" as manufactured by Reef Industries, Inc., Houston, TX, or approved equal.

2.02 LOCATION WIRE

A. Location (Tracer) Wire for Polyvinyl Chloride and HDPE pipe (and other pipe where noted in the drawings or identified in special conditions)

Location wire shall be a direct burial #12 AWG Solid (.0808 inch diameter), 21% conductivity annealed copper-clad high carbon steel strength tracer wire, 380# average tensile break load, 30-millimeter high molecular weight-high density blue polyethylene jacket complying with ASTM D1248, 30-volt rating. The wire shall be contiguous except at test stations, valve boxes, and where splicing is required. All splices shall be encased with a 3M-Gel Pack model No. 054007-09053. Wire insulation shall be highly resistant to alkalis, acid and other destructive agents found in soil.

B. Location Wire shall be from Copperhead Industries, LLC, part number 1230B-HS or approved equal.

C. If directional drilling is used for this project please refer to Specification 02458 for the product description of location wire to be used with the directional drilling

2.03 RESTRAINED JOINT MARKING TAPE

- A. Joint restraint tape is specifically to warn Water Company workers/contractors that the water main is joint restrained. It is not to be used in place of regular marking tape.
- B. Restrained Joint Marking Tape (for with mains that are restrained joint as directed by the Engineer) shall be polyethylene 4 millimeters thick and 2 ¹/₂-inches wide with blue lettering on white background color and imprinted with the words "RESTRAINED JOINT" every 2 feet. The tape shall have an adhesive backer. The tape shall be highly resistant to alkalis, acid and other destructive agents found in soil.
- C. Restrained Joint Gasket indicator tape shall be part number 515401-010 manufactured by St. Louis Paper & Box Company located at 3843 Garfield, St. Louis, MO 63113 or approved equal.

PART 3: EXECUTION

3.01 INSTALLATION OF IDENTIFICATION TAPE

- A. Install the identification tape with all buried potable water lines in accordance with the manufacturer's installation instructions and as specified.
- B. Install identification tape one foot above the top of the pipe.

3.02 INSTALLATION OF LOCATION (TRACER) WIRE

- A. Install location wire with buried water lines in accordance with the manufacturer's installation instructions and as specified in Contract Documents.
- B. Install the location wire directly on top of the buried pipe.
- C. In all pipe installations, loop the location wire up into the valve boxes for connection to a locating device. The wire shall be one continuous piece from valve box to valve box up to 1250 feet maximum.

3.03 INSTALLATION OF RESTRAINED JOINT MARKING TAPE

A. Install the joint marking tape by adhering directly to the pipe as it is installed. The marking tape shall be installed along the entire length of pipe, including around the circumference of the bells of all fittings and valves. The pipe must be free of any foreign matter along the surface of the pipe for the marking tape installation. If clear polywrap is used, the restrained joint tape can be applied on the top of the pipe so long as it is visible. Otherwise the joint marking tape shall be applied on top of the polywrap and secured so the tape is not shifted by backfilling.

B. The tape does not adhere in wet or cold conditions. The tape should be stored in temperatures above 50°F until the time of application. The pipe must be free of frost and moisture along the surface of the pipe receiving the tape.

END OF SECTION

SECTION 02610

PAVING AND SURFACING

PART 1: GENERAL

1.01 DESCRIPTION

- A. Provide all labor, tools, material and equipment to replace pavement, traffic control loops, pavement stripping, curbs, drives and walks that have been damaged or disturbed during the course of the work, all as specified in contract documents, as directed by the Engineer, or as required by local, state, or federal regulations. Placement will be at least equal to the type of pavement, curb, drive, or walk which existed before the work began and to the satisfaction of the Engineer.
- B. Furnish all labor, tools, material, and equipment necessary to spread and roll and/or tamp temporary bituminous pavement, complete, in place, and maintain the same all as specified or as directed by the Engineer.
- C. During the entire period of construction of the project, keep all streets, curbs, drives and walks in clean, usable, and safe conditions for public use. Keep the work area free from accumulations of waste material, rubbish and other debris resulting from the Work. Clean all roadways daily. Sweep, scrape, shovel or use whatever other approved means, including mechanical pickup sweeper that may be necessary to clean and maintain the roadways to the satisfaction of Owner and the agency having jurisdictional control over said road.
- D. Before final acceptance and after any trench settlement has been corrected to the satisfaction of the Engineer, replace pavement, curbs, drives and walks designated by the Engineer with the type of replacement specified.

PART 2: PRODUCTS

2.01 MATERIALS

A. Furnish materials of construction for traffic control loops, pavement stripping, paving, curbing, and surfacing in accordance with applicable Federal, State and local standards. If there are no applicable standards, use materials which will produce a result that is at least equal to the type which existed before the work began and that is to the satisfaction of the Engineer.

PART 3: EXECUTION

3.01 INSTALLATION

- A. Saw or line cut the existing pavement, where necessary, as required by local, State or Federal regulations. The edges of the face of the old pavement or base shall be left vertical. Trim ragged edges so as to provide a substantially straight line juncture between the old and new surfaces.
- B. Place the pavement replacement so as to conform in grade to the existing streets, drives or sidewalks. The type of pavement replacement shall be as

VDH Approved 09/2020 – Prince William District 02610-1 shown on the pavement replacement details in accordance with applicable Federal, State or local standards. If there are no such applicable standards, replacement will be made to the satisfaction of the Engineer.

- C. Roll and tamp in place a 2-inch thick (minimum) course of bituminous material over trenches where temporary pavement is ordered. Remove temporary pavement prior to the placing the permanent pavement. The cost shall be included in the contract price. The finished temporary surface shall be flush with the adjacent undisturbed surface. Maintain the temporary bituminous surface until the temporary surface is replaced.
- D. Before the completion of each day's work, in traveled areas, pave the pipe trench with 6 inches of stabilized base, unless another method of pavement restoration is required by the authorized governing body. Place final paving over the stabilized base, overlap each side of the trench a minimum of 6 inches, and feather to meet the existing pavement; unless another method of pavement restoration is required by the authorized governing body. Place final pavement at least 20 days and not more than 45 days after the backfilling has been completed, unless otherwise directed by the Engineer.
- E. Instead of temporary paving, the use of steel roadway plates may be required if an excavation within traveled areas is subject to repeated access prior to backfill/final paving. The use of steel roadway plates shall be in strict accordance all applicable regulations with the Federal, State, County, and/or Local Agency having jurisdiction. Properly secure the steel roadway plates so that they will not be "dragged" from place by a braking truck or "pushed" from place by a snowplow. Submit load bearing calculations, when requested by the Engineer, sealed by a Professional Engineer who is licensed to practice in the applicable State. Calculations must demonstrate that the steel roadway plate is properly designed and installed to accommodate HS-20 vehicular loadings based upon plate dimensions (L x W x T), steel strength, and the size of the excavation (L x W) to be protected.

3.02 MAINTENANCE

A. Following the certification of completion by the Engineer, maintain the surfaces of curbs and gutters, paved surfaces and sidewalks for a period of one year thereafter, or for such greater period as may be required by Federal, State or local authorities. Supply all material and labor required for such maintenance. The work shall be done in a manner satisfactory to the Owner at no additional cost to the Owner.

END OF SECTION
CONCRETE CURBS, DRIVES, AND SIDEWALKS

PART 1: GENERAL

1.01 SCOPE

The work under this section shall include the installation of all concrete curbs, sidewalks, and drives. Installation will include new installations as required on the drawings, and replacement of all curbs, drives and sidewalks damaged or removed incidental to construction. Adhere to most stringent requirements between local regulations and this specification concerning concrete installations for work performed on property owned by others (the municipality or private owners other than American Water).

PART 2: PRODUCTS

2.01 CONCRETE

- A. All concrete shall conform to the following: ASTM C-150 Type I Portland cement, Class A 3,000 psi; design mix, with a 4-inch <u>+</u> air-entrained slump ready mixed in accordance with ASTM C-94.
- B. Aggregate shall conform to ASTM C-33, which is clean, hard, durable, screened, crushed stone or gravel. The aggregate shall contain no cheat.

2.02 REINFORCEMENT

As needed to meet or exceed existing conditions or as specified in these contract documents.

2.03 CURING COMPOUND

Curing compound shall conform to the specifications of ASTM C-309, Type II, clear, and shall consist of a practically colorless impervious liquid which will thoroughly seal the surface of the concrete and will not impart a slippery surface thereto. The quality and the quantity to be used shall be approved by the Engineer. The use of any material which would impart a slippery surface to the concrete or alter its natural color will not be permitted. The colorless, impervious compound shall contain not less than twenty-five percent (25%) solids. Admixtures applied to concrete with reinforcing steel require review and approval by the Engineer before use.

2.04 PROTECTION

Immediately upon finishing the concrete, the concrete shall be completely covered with plastic, or alternate approved by the Engineer. Canvas or wetted straw will not be allowed as alternate coverings for curing.

PART 3: EXECUTION

3.01 CURBS

- A. All base for the installation shall be thoroughly compacted to support curb installation. Expansion joints should be provided at a minimum of every 12 feet.
- B. All new curb installations shall be as shown on the drawings, and as detailed on the detail sheets.
- C. All replacement curbs shall be of the same type and thickness as the curb and gutter which it abuts. The grade of the restored curb and gutter shall conform with the grade of the existing adjacent curb and gutter, and installed to insure there is no ponding of water.

3.02 DRIVEWAYS

- A. All base for the installation shall be thoroughly compacted and leveled to support the new and replacement installations without settlement. Expansion joints should be provided at a minimum of every 30 feet.
- B. All new driveways shall be installed as shown on the plans, and as detailed on the detail sheets.
- C. All permanent restoration of driveways shall conform to the construction as originally placed and to the original lines and grades, unless directed otherwise by the Engineer.
 - 1. No patching of concrete driveway areas will be allowed between joints or dummy joints.
 - 2. All joints shall be saw cut.
 - 3. In no case shall the thickness of the driveway be less than 4 inches, with 6x6x6/6 woven wire mesh.

3.03 SIDEWALKS

- A. All base for the installation of sidewalks shall be thoroughly compacted and leveled to support the new and replacement installations without settlement. Expansion joints should be provided at a minimum of every 30 feet.
- B. All new sidewalks shall be installed as shown on the plans and as detailed on the detail sheets.
 - 1. Sidewalks shall have a minimum thickness of four inches, with 6x6x10/10 wire mesh.
 - 2. All sidewalks shall slope 1/4 inch per foot across the width of the walk toward the street.
 - 3. The finish shall be a broom finish at right angles to the walkway.

- 4. Dummy expansion grooves shall be marked on the sidewalk at five-foot intervals. The grooves shall be ½-inch deep by 3/8-inch in width.
- 5. Sawed grooves will not be permitted.
- C. All permanent restoration of sidewalks shall conform to the manner of construction as originally constructed and placed (brick, block or stone).
 - 1. When concrete sidewalks are replaced, the replacements shall match the existing line and grades, and width.
 - 2. All replacement work shall meet the requirements of new sidewalk construction. No patching will be allowed between joints or dummy joints.
 - 3. If a curing compound is employed, it shall be applied per the manufacturer's direction and at a recommended rate of application. If unknown, it shall be applied at 1 gallon (3.79 liters) per 200 square feet (18.58 square meters) for each coat. Surfaces damaged by construction operations during curing shall be resprayed at the same rate.

3.04 PAVED SIDE DITCH

- A. All base for the installation shall be thoroughly compacted and leveled to support the new and replacement installations without settlement.
- B. All new side ditch shall be installed as shown on the plans, and as detailed on the detail sheets.
- C. All permanent restoration of side ditch areas shall conform to the construction as originally placed and to the original lines and grades in accordance with the current appropriate state transportation department guidelines.
 - 1. No patching of concrete side ditches will be allowed between joints or dummy joints.
 - 2. All joints shall be saw cut.

3.05 PROTECTION

All concrete work shall be protected by barricades, lights, etc. to protect the concrete until set-up.

END OF SECTION

LAWN RESTORATION

PART 1: GENERAL

1.01 DESCRIPTION

Restore and replace shrubbery, fencing, or other disturbed surfaces or structures to conditions equal to that before the work began and to the satisfaction of the Engineer.

PART 2: PRODUCTS

2.01 TOPSOIL

Topsoil shall not contain more than 40% clay in that portion passing a No. 10 sieve. Topsoil shall contain between 5% and 20% organic matter as determined by loss on ignition of samples oven-dried to constant weight at 212 °F.

2.02 FERTILIZER

Fertilizer shall be lawn or turf grade 12-12-12.

2.03 SEED AND SOD

A. Lawn Areas

Seed areas where lawns are or have been regularly maintained, whether residential, commercial or office areas, with the following mixture or a mixture as required by the Soil Conservation District or other governing authority. (Percentages are by weight.)

- 1. 20% Annual Ryegrass (Lolium multiflorum)
- 2. Remainder to be specified depending on time of year, regulatory requirements and location.

Where sod is required it shall be green, freshly cut, and of good quality with grass free from all noxious weeds. It shall contain all the dense root system of the grass and shall not be less than 1-1/2 inches thick.

B. All Other Areas

Seed all other areas with the following mixture:

- 1. 50% Perennial Ryegrass (Lolium perenne)
- 2. Remainder to be specified depending on time of year, regulatory requirements and location.

2.04 MULCH

Mulch shall be straw reasonably free of weed seed and foreign materials which may affect plant growth. Other materials may be used if approved by the Engineer.

2.05 ASPHALT EMULSION

Emulsion shall be non-toxic to plants and shall conform to AASHTO M140 or AASHTO M208.

PART 3: EXECUTION

3.01 PREPARATION OF SEED BED

A. <u>Topsoil Areas</u>

Removed, store, and use suitable topsoil available from the excavated material to backfill the top 4 inches of the excavation. Remove and dispose of all imported granular fill, grass, weeds, roots, sticks, stones, and other debris 1-inch or greater in diameter. Bring the topsoil to the finished grade by raking.

B. Non-Topsoil Areas

When there is insufficient topsoil available from the site excavated materials, furnish 4 inches of topsoil to be used as a seed bed in lawn areas as described in Part 2.03, Paragraph A of this Specification Section or clearly marked as lawn areas on the plans.

The trench backfill may be used as a seed bed, where approved by the Engineer or in areas clearly marked on plans that are not considered lawn areas. After the backfill has been given a reasonable time to settle, grade it off to the finished grade and harrow to a depth of 3 inches. Remove and dispose of all grass, weeds, roots, sticks, stones and other debris 1 inch or greater in diameter. Carefully bring the topsoil to the finished grade by raking.

3.02 FERTILIZING

Apply fertilizer uniformly to all areas to be seeded at the rate of 1 pound per 100 square feet in topsoil and 2 pounds per 100 square feet in non-topsoil. Disk, harrowed, or raked the fertilizer thoroughly into the soil to a depth of not less than 2 inches. Immediately before sowing the seed, rework the surface until it is a fine, pulverized, smooth seed bed varying not more than 1 inch in 10 feet.

3.03 SEEDING

Seed immediately after preparation and fertilization of the seed bed. Mix the seed thoroughly and sow it evenly over the prepared areas at the rate of 3 pounds per 1,000 square feet. Sow the seed dry or hydraulically. After sowing, rake or drag the area to cover the seed to a depth of approximately 1/4 inch

Sod all areas with slopes greater than 10%.

3.04 SODDING

Sod all areas as noted in the drawings. As a minimum, sod shall be fibrous, well rooted approved grass type. The grass shall be cut to a height of less than three (3) inches. Edges of sod shall be cleanly cut, either by hand or machine, to a uniform thickness of not less than one and one-half $(1-\frac{1}{2})$ inches, to a uniform width of not less than sixteen (16) inches, and in strips of not less than three (3) feet in length. Sod shall be free from all primary noxious weeds as defined by the applicable State Seed Law.

Lay sod with tight staggered joints. On slopes, start placement at the foot of the incline. Use wood pegs driven flush to hold sod in place on slopes 4:1 or greater. Use two wood pegs per strip of sod. Roll the sod lightly after placement. Fill any open joints with topsoil and/or sod.

3.05 MULCHING

Place mulching material evenly over all seeded areas within 48 hours of seeding. Place mulch at the rate of approximately 2 tons per acre, when seeding is performed in recognized growing season and at the approximate rate of 3 tons per acre when seeding is performed in a recognized non-growing season if applicable.

3.06 EMULSION

Keep mulching materials in place with asphalt emulsion applied at a minimum rate of 60 gallons per ton of mulch or by other methods approved by the Engineer. When mulch is displaced, immediately repair any damage to the topsoil and fertilizer, re-seed, and re-mulch per the requirements of this Specification Section.

3.07 MAINTENANCE

Carefully maintain, tend, and water all seeded and sodded areas necessary to secure a good turf. Fill, grade, and reseed or re-sod all areas that have settled. Maintain the condition of the sodded areas for a period sufficient for the grass to root into the topsoil. Maintain the condition of the seeded areas in accordance with the requirements of this Specification Section for a period of one year from the date of final completion.

END OF SECTION

PIPE BURSTING

PART 1: GENERAL

1.01 SCOPE AND METHODS

- A. The Contractor shall furnish all labor, materials, tools and equipment as necessary to rehabilitate existing water mains using a pipe bursting system to provide the Owner with a complete, finished water main installation to include restoration of service connections.
- B. The pipe bursting tool shall be designed and manufactured to force its way through existing pipe materials by fragmenting the pipe and compressing the old pipe sections into the surrounding soil as it progresses. The bursting unit shall generate sufficient force to burst and compact the existing pipeline.
- C. The bursting system shall be either static or pneumatic but have sufficient strength to burst the pipe, repair clamps and couplings that may be encountered.

1.02 RELATED SECTIONS

Section 01300 - Submittals

Section 02200 - Excavation, Backfilling and Compaction

Section 15000 - Piping - General Provisions

Section 15020 - Disinfecting Pipelines

- Section 15131 Piping Specialties
- Section 15200 Service Lines (Contractor Furnished)

Section 15205 - Service Lines (Owner Furnished)

1.03 QUALITY ASSURANCE

- A. For a pipe bursting system to be considered commercially proven, a minimum of 100,000 linear feet of successful water main rehabilitation in the United States must be documented to the satisfaction of the Owner.
- B. The Contractor shall be certified by the pipe bursting system manufacturer as a fully trained user of the pipe bursting system and shall have performed a minimum of 10,000 linear feet of pipebursting. The Contractor shall hold the Owner whole harmless in any legal action resulting from patent infringements.
- C. The system equipment operator(s) shall be trained to operate the specific pipe bursting equipment to be utilized. Only the contractor's employees who have been trained and certified by the pipebursting equipment manufacturer shall be allowed to operate the equipment during the project. The Contractor shall provide certifications of training and proficiency in the use of the equipment. Pipe bursting operations shall be under the constant direction of a trained and certified supervisor who shall remain on site and be in responsible charge throughout the pipe installation.

D. If polyethylene pipe is used as the replacement pipe, the polyethylene pipe jointing shall be performed by personnel trained in the use of butt-fusion equipment and the specified methods for installing new pipe connections. Personnel directly involved with installing the new pipe shall receive training in the proper methods for handling and installing polyethylene pipe in advance of the work performed. Such training shall be conducted by a qualified representative of the fusion equipment manufacturer. Installation of other connecting polyethylene pipe materials shall be performed by personnel qualified by the specific product manufacturer.

1.04 PROFILES AND TOPOGRAPHY

A. Contours, topography and profiles of the ground as may be shown on the Contract Drawings are believed to be reasonably correct but are not guaranteed to be absolutely so and are presented only as an approximation. It is the Contractor's responsibility to verify all elevations required to successfully complete the installation.

1.05 SUBMITTALS

- A. The Contractor shall submit the following items for review and approval in accordance with the requirements of Section 01300. Approval of the submittals shall be obtained prior to ordering pipe materials and/or the start of the pipe replacement process.
 - 1. Certifications of training by the pipe bursting system manufacturer stating that the operators have been fully trained in the use of the pipe bursting equipment by an authorized representative of the equipment manufacturer.
 - 2. Certifications of training by the pipe fusion and pipe tapping equipment manufacturers that the operators have been fully trained in the use of the fusion and tapping equipment respectively by an authorized representative of the equipment manufacturer.
 - 3. Method of construction with detailed drawings and written descriptions of the entire construction procedure to burst the existing pipe, pipe fusion techniques, non-fusion pipe joining techniques, insert the replacement pipe and connections to water services, fire hydrants and intersecting water mains. Drawings shall show, at a minimum, excavation locations, access pits, dimensions, shoring, method of dewatering, adjacent utilities and traffic control.
 - 4. Locations, sizes and construction methods for service reconnection pits.
 - 5. Methods of construction, reconnection and restoration of existing water services, fire hydrants and intersecting mains.
 - 6. Plans and procedures for supplying temporary water service, if required.

- 7. Manufacturer's literature, MSDS sheets, and NSF Certification for lubricant to be utilized during pipe installation.
- 8. Contingency plans for the following potential conditions:
 - a. Unforeseen obstruction(s) causing burst stoppage, such as unanticipated change(s) in host pipe material, repair section(s), concrete encasement(s) or cradle(s), or unforeseen changes in direction.
 - b. Soil heaving or settlement.
 - c. Damage to adjacent utilities.

Contingency plans shall include providing a contact person who can be contacted immediately to respond to problems that may arise including issues with temporary water services.

- B. Pre-bursting and post-bursting television inspection reports including information about the capabilities of the CCTV unit (swivel, tractor type, etc.) to demonstrate compliance with section 3.04D.
- C. Pipe submittals shall include, but are not limited to, the following:
 - 1. Pipe lengths, design details, joint details, etc.
 - 2. All welding or fusion procedures to be used in fabrication of the different pipe materials and installation methods.
 - 3. Certified records for hydrostatic testing of all pipe materials to be used.
 - 4. An affidavit stating that all pipe materials furnished under this section have been manufactured in the United States of America and comply with all applicable provisions of referenced AWWA standards.
- D. Time schedule for completing the installation of the replacement pipe and return of the main to normal service.

1.06 JOB CONDITIONS

- A. Planned nighttime work is expressly prohibited and will not be allowed unless prior approval is granted by the Owner and all regulatory agencies having jurisdiction.
- B. Unless expressly permitted by the Engineer in writing, all pipe bursting operations shall be accomplished during daylight hours and shall not begin after the hour preestablished as the latest starting time that will allow completion during daylight hours. The Contractor shall provide a Work Plan submittal indicating its proposed hours of operation and length of workweek. All work plans shall be subject to

compliance with applicable regulatory requirements for construction activities and any off-site impacts.

- C. When hazards of night time work are carefully considered and determined to be insignificant, night time work may be allowed only to complete a properly planned installation, and only if allowed by the local jurisdiction and, in the opinion of the Engineer, the delay was caused by reasonably unavoidable circumstances, and that such night time work is necessary to avoid placing an undue economic hardship on the Contractor.
- D. In emergency situations, or where delay would increase the likelihood of a failure, nighttime work may be allowed to complete a delayed installation. The Contractor must notify the Engineer of such situations as soon as they become apparent.

1.07 COORDINATION OF WORK

A. Coordinate connections to existing pipelines that require shutdown of Owner facilities. Owner will designate the time for these connections that could involve work during evenings, nights, Saturdays, Sundays, or holidays. Method of connection and designated times are to cause the least amount of disruption to Owner's water service to its customers. The cost for connections is to be included in the contract price. No contract price adjustment will be allowed for overtime, premium time, or other related costs.

1.08 USE OF EXISTING WATER SYSTEMS

A. All use of existing water systems during construction by the Contractor shall be with the approval and direction of the system Owner and its representatives. The Contractor shall be responsible for all permits, fees, temporary piping, temporary meter rental/provisions, temporary backflow preventer rental/provision and other water utility requirements for supplying water during construction. The Contractor shall use the existing water system only at locations, times and conditions as set forth by the system owner or its representatives.

PART 2: PRODUCTS

2.01 PIPE

Unless otherwise specified in the Contract Documents, pipe installed by pipe bursting shall be high density polyethylene pipe (HDPE) or ductile iron pipe (DI), furnished in the same nominal diameter as the existing main.

- A. Polyethylene Pipe
 - 1. High Density Polyethylene (HDPE) Pipe, AWWA C-906 compliant, NSF 61 Standard Listed, and furnished in fifty (50) foot lengths.
 - 2. Polyethylene pipe shall be furnished with an outside diameter conforming to ductile iron pipe sizes. Minimum thickness of HDPE pipe shall be determined by the Contractor's calculations, but shall not be less than DR

11 when measured in accordance with ASTM D-2122. The minimum pressure rating will be 200 psi.

- 3. The pipe shall contain no recycled compound except for rework material generated in the manufacturer's own plant that has the same cell classification as the material to which it is being added. The pipe shall be homogeneous throughout and free of visible cracks, holes, voids, foreign inclusions, or other defects that may affect the wall integrity.
- 4. Permanent identification of water piping service shall be provided by coextruding longitudinal blue stripes into the pipe outside surface. The striping material shall be the same material as the pipe material except for color. Stripes printed or painted on the outside surface shall not be acceptable.
- 5. All polyethylene pipe and fittings shall be made of a high-density polyethylene pipe compound with extra high molecular weight that meets the requirements for Type III, Grade P34 Polyethylene material as defined in ASTM D-1248, latest revision.
- 6. Pipes shall be jointed to one another and to polyethylene fittings by thermal butt-fusion or by socket fusion in accordance with ASTM D-3261.
- 7. Joining of pipe sections shall be performed in accordance with the procedures recommended by the pipe manufacturer. Joints between pipe sections shall be smooth on the inside and internal projection beads shall not be greater than 3/16-inch.
- 8. The tensile strength at yield of the butt-fusion joints shall not be less than the pipe. A specimen of pipe cut across the butt-fusion joint shall be tested in accordance with ASTM D-638.
- 9. Polyethylene pipe shall be joined to ductile iron pipe by the use of flange adapters and back-up rings. Flange adapters shall be butt fused to the polyethylene carrier pipe. The face of the flange adapter shall have a serrated sealing face to assist in holding the flange gasket in place. Flange gaskets shall be full-faced neoprene. Back-up rings shall be Class "D" steel ring flanges in accordance with AWWA C207. Flange bolts must span the entire width of the flange joint, and provide sufficient thread length to fully engage the nut.
- B. Ductile Iron Pipe
 - 1. Utilize ductile iron pipe equipped with low profile flexible restrained joints such as American Cast Iron Pipe Company Flex-Ring, US Pipe and Foundry TR Flex, or Griffin Pipe Products Snap-Lok. Gripping push-on joint gaskets or restrained joint gaskets are not permitted.
 - 2. Ductile iron pipe shall meet the requirements of AWWA/ANSI C151/A21.51.

- 3. Ductile iron pipe shall be lined with cement mortar per AWWA/ANSI C104/A21.4. Pipe shall be furnished with a standard asphalt external coating approximately one mil thick in accordance with AWWA/ANSI C151/A21.51.
- C. Inspection of Pipe
 - 1. All pipe and fittings used in the work may be factory inspected by a recognized agency engaged by the Owner. The Contractor shall inform the Owner and the inspection agency of the name and address of the manufacturing plant or other sources of materials to be used in the work and shall coordinate with the manufacturer to assure that the inspection agency has access at the manufacturer's plant and adequate assistance and notice so that each item may be examined. All reports will be made to the Owner and the cost of the services of the inspection agency will be borne by the Owner. Such inspection by the Owner shall not relieve the Contractor of his responsibility to furnish materials in accordance with the applicable standards.

2.02 EQUIPMENT

- A. Pipe bursting shall be accomplished by either static or pneumatic methods. The Contractor shall select the bursting method after considering the existing soil conditions, existing pipe material, length of installation, installation depth, proximity to existing utilities, and new pipe diameter. Static methods shall utilize the TT Technologies, Inc. Grundoburst pipe bursting system or approved equal. Pneumatic methods shall utilize the TT Technologies, Inc. Grundocrack pipe bursting system or equal.
- B. Static Pipe Bursting Equipment:
 - 1. The bursting action of the tool shall increase the external dimensions sufficiently to cause breakage of the pipe while at the same time expanding the surrounding ground. The bursting action shall not only break the pipe, but also create the void of sufficient size into which the burster can be statically pulled which enables forward progress to be made. Simultaneously, the new pipeline, directly attached to the expander, shall also move forward.
 - 2. The static pulling frame shall be telescopic in design to allow the cutting head to release at the termination of the pull.
 - 3. Quick lock bursting rods are required to guarantee snap lock connections. Threaded bursting rods will not be permitted.
 - 4. The unit must maintain automatic thrust and pull back, and be capable of pipe bursting in two directions from the same excavation.

- C. Pneumatic Pipe Bursting Equipment:
 - 1. The pipe bursting tool shall be assisted through the line to be replaced by a winch located at the receiving excavation. The bursting unit shall pull the replacement pipe with it as it moves forward. The bursting head shall incorporate a shield/expander to prevent collapse of the hole ahead of the replacement pipe insertion. The pipe bursting unit shall be remotely controlled.
 - 2. The burster shall have its own forward momentum while being assisted by winching. The burster must be matched to a constant tension hydraulic winching system.
 - 3. The winch shall be hydraulically operated and provide constant tension throughout the operation. The winch shall be fitted with a direct reading load gauge to measure the winching load. The winch shall be fitted with a device to automatically disengage when the load exceeds a preset maximum load.
 - 4. The constant tension winch shall supply sufficient cable in one continuous length so that the pull may be continuous between approved winching points.
 - 5. The winch, cable and cable drum must be provided with safety cage and supports so that they may be operated safely without injury to persons or property. The Contractor shall provide a system of guide pulleys and bracing at the exit pit to minimize cable contact with the existing line between launch and exit pits.
 - 6. Supports for trench shoring shall remain completely separate from the winch boom support system and shall be designed such that neither the pipe nor the winch cable shall be in contact with them.

PART 3: EXECUTION

3.01 SITE DISTURBANCE AND SOIL EROSION

- A. Sediment barriers shall be constructed as shown on the Drawings or where directed by the Engineer. All soil erosion and sediment control work shall be done in accordance with the Standards for Soil Erosion and Sediment Control for the location where the work is performed. Contractor shall maintain sediment barriers until the project is deemed complete.
- B. The Contractor shall be responsible for the preservation of all existing trees, plants, and other vegetation that are to remain within or adjacent to the construction site and shall also be responsible for protecting existing concrete curb, fence, utilities, and other structures that are located within or adjacent to the construction site.

3.02 LAUNCHING AND RECEIVING PITS

- A. The location and number of launching and receiving pits shall be proposed by the Contractor and approved by the Engineer prior to excavation. The pits shall be located such that their number is minimized and the footage of new pipe installed in a single pull is maximized.
- B. Walls of pits shall be adequately braced or reinforced to resist the horizontal forces generated during the bursting operation.
- C. All work is to be completed within existing rights-of-way or Water Company easements. The Contractor shall be responsible for confirming the limits in consultation with the Engineer.
- D. Any damage to adjacent properties that are not part of this work shall be repaired and restored to their original condition at the Contractor's expense.

3.03 UTILITY INVESTIGATIONS

- A. The depth, location, and size of all existing underground facilities in the vicinity of the proposed installation shall be determined by the Contractor before starting any construction. The Contractor shall be held completely and solely responsible for any damages incurred. The kinds, locations and sizes of the existing underground utilities which may be shown on the Contract Drawings are intended only as a guide to the Contractor and are not guaranteed to be even approximately correct. The owners of all existing utilities within the project limits shall be notified prior to the start of construction by the Contractor.
- B. All buried utilities adjacent to the existing water main shall be reviewed, and where necessary shall be excavated to confirm their proximity to the pipebursting and relieve transient loading during the insertion operation. If any utilities are within 24-inches of the pipe to be burst, the Contractor shall excavate a pit at the location to check clearance. If adequate clearance does not exist between the existing water line and the affected utility, the Contractor shall modify the limits of pipe to be installed by pipebursting.
- C. At any utility crossings within 6-inches of the existing water main to be burst, soil shall be excavated and removed to relieve loading during the bursting operation.
- D. Any known concrete encasements shall be excavated and broken out prior to the bursting operation to allow the steady and free passage of the bursting head. All inline valves and fittings shall be removed prior to the bursting operation.
- E. Prior to the start of work, the Contractor shall take photographs or video of any improved surface in the areas of the access pits and areas over which the pipe bursting equipment will pass. This documentation shall be submitted to the Engineer on a DVD or CD using conventional formatting.

3.04 TELEVISION INSPECTION

- A. Pre-installation internal television inspections shall be performed as necessary to locate obstructions or existing fittings that would inhibit the bursting process. The locations of all existing connections shall be noted.
- B. Television inspection shall consist of a closed-circuit television and a DVD recorder. The equipment operator shall be thoroughly experienced in the use of the equipment and shall be required to describe the various portions of water mains being televised including the location, distance (station), size, and limits of each section recorded.
- C. The equipment used for the televised inspection shall be specifically designed for pipeline inspection including its own lighting system which shall be capable of providing the necessary light level to produce pictures of excellent quality. The television camera shall be capable of producing a picture quality which corresponds to a continuous 600-line resolution picture showing the entire periphery of the pipe. The pipe interior shall be clearly visible without static interference of any kind. The television camera shall be equipped with a rotating lens capable of rotating a full 360° (pipe 6" and above).
- D. Following completion of the television inspection work, the Contractor shall turn over to the Engineer the completed continuous pre- and post-installation DVD's with voice narration. The DVD's must contain a distance indicator throughout the inspection in order to locate any sections found to be defective.
- E. All television inspection shall be done in the presence of the Owner and the Engineer. The Contractor shall notify the Owner and the Engineer at least 48 hours prior to performing the television inspection.

3.05 BURSTING AND PIPE INSTALLATION

- A. The Contractor shall be responsible for providing a Maintenance of Traffic (MOT) Plan to the Engineer and local traffic law enforcement agency for review. The MOT Plan shall show the location of all barricades, signs, devices and alternate routes for local traffic and pedestrian safety. Erection of the appropriate safety and warning devices in accordance with the USDOT "Manual of Uniform Traffic Control Devices" (MUTCD) shall be completed prior to beginning work and maintained until all construction is completed and the site restored.
- B. The Contractor shall specifically note that street intersections must remain open during the pipe installation operation, or traffic detours implemented. This may require the Contractor to install a temporary sleeve across the street intersections through which the pipe can be pulled or to construct a temporary bridge for the pipe over the intersections. No additional payment will be made for temporary structures required in order to permit access through street intersections or the implementation of traffic detours.

- C. Service connections and fire hydrant laterals shall be disconnected from the existing main prior to the start of bursting operations. If hydrants are out of service, they shall be appropriately bagged.
- D. Do not exceed manufacturer's recommended insertion rate or force at any time. Maintain logs to verify that rate and force did not exceed permissible values.
- E. If required by field conditions, use an approved, environmentally acceptable lubricant to ease installation friction. Match lubricants to soil and insertion conditions.
- F. Contractor shall haul, string, assemble restrained joint pipe, joint air test and hydrostatically test the pipeline in one section, unless otherwise approved by Engineer. Ductile iron pipe may be assembled by either the cartridge method or the assembled line method. The Contractor shall provide adequate site security and shall be responsible for the integrity of the pipe until after the installation and acceptance of the work by the Owner.
- G. All assembled pipe sections shall be securely plugged at the end of each workday. The pipe interior is to be protected at all times against dirt, dust, pipe cuttings, debris, animal access, and other sources of contamination.
- H. Contractor shall provide adequate support rollers for the pipeline during installation. The rollers and cradles shall be of a type that will prevent damage to the pipe and will be of sufficient number, as recommended by pipe manufacturer, to prevent over stressing due to sag bends during the installation procedure. The pipe shall be supported at all times, including installation, to maintain a free stress arc which limits pipe bending and internal hoop stresses to within manufacturer's limits.
- I. Unless otherwise noted in the Contract Documents, settlement or heaving of the ground surface during or after construction will not be allowed. The Contractor is solely responsible for the costs for repairing any surface heaving or settlement unless specified otherwise in the Contract Documents.
- J. Pipe which is not properly protected and supported and shows indications of excessive stressing, gouges, cuts, abrasions or other damage which may affect the operational performance intended for the pipe, as recommended by pipe manufacturer, shall be removed from the site and replaced at no additional cost as directed by the Owner or Engineer.
- K. The pipe shall be pulled sufficiently to allow for visual examination of the first three feet of pipe pulled in the receiving pit. If, in the opinion of the Engineer, the exposed pipe shows excessive cuts, gouging or abrasions, the Contractor will be required to pull additional pipe until the problem is no longer in evidence. Alterations required in the receiving pit and other changes to perform this work shall be done at the expense of the Contractor.

3.06 PRESSURE TESTING AND LEAKAGE

A. Prior to insertion, an allowable leakage test shall be performed on the full length of pipe after all sections have been joined, welded or fused in accordance with ANSI/AWWA C600, of latest revision and as described in Specification Section 15030. A hydrostatic pressure test shall also be performed on the installed pipe in accordance with ANSI/AWWA C600, latest revision, and as described in Specification Section 15030.

3.07 CONNECTION TO EXISTING MAINS AND SERVICE LINES

- A. If PE pipe is used, allow inserted pipe to rest (normalize) for a minimum period of 4 hours before cutting/trimming and connecting to existing mains.
- B. Where polyethylene pipe is connected to ductile iron fittings or valves, a HDPE flange adapter with a steel back-up ring shall be fused to the end of the pipe and the connection made with bolted flange components. All connections of new pipe to the existing water system shall be restrained.
- C. If PE pipe is used, Polyethylene and flange gasket will undergo some compression. Therefore, the flange bolts shall be retightened one hour after the initial assembly, and a second time at least four hours after the second tightening.
- D. Reconnection of existing services shall be as specified in Specification Section 15200 or 15205. For PE pipe, service connections installed on new polyethylene pipe shall be accomplished by the side wall fusion method in accordance with the manufacturer's printed instructions.

3.08 DISINFECTION

A. The installed pipe shall be disinfected as described in Specification Section 15020.

END OF SECTION

CAST-IN-PLACE CONCRETE

PART 1: GENERAL

1.01 SCOPE OF WORK

Provide concrete for thrust blocking, manhole bases, pipe encasement, curbs, sidewalks and pavement in accordance with this Specification Section.

PART 2: PRODUCTS

2.01 MATERIALS

- A. <u>Portland Cement</u> shall be Type I or Type III and conform to "Specification for Portland Cement" ASTM C150.
- B. <u>Air-Entraining Agent</u> from approved manufacturer shall be added in accordance with manufacturer's directions to the normal Portland cement to entrain 4½ percent air ± 1 percent with all other ingredients and strength as specified. Airentraining admixtures shall conform to "Specifications for Air-Entraining Admixtures for Concrete" ASTM C260.
- C. <u>Concrete Aggregates</u> shall conform to "Specifications for Concrete Aggregates" ASTM C33. Coarse aggregates shall be a maximum of 1¹/₂ inches in size in footings and plain concrete. Pea gravel shall be used for sections 3 inches or less in thickness.
- D. <u>Water</u> used in mixing concrete shall be clean and free from injurious amounts of oils, acids, alkalis, organic materials, or other deleterious substances. In effect, the water used shall be potable water.
- E. <u>Reinforcing Bars</u> shall be billet steel grade (60,000 psi minimum yield) conforming to the requirements of ASTM A615, Grade 60. Reinforcing bars shall be new stock, free from rust, scale, or other coatings that tend to destroy or reduce bonding.
- F. <u>Welded Wire Mesh</u> shall conform to "Standard Specification for Carbon-Steel Wire and Welded Wire Reinforcement, Plain and Deformed, for Concrete" ASTM A1064.
- G. <u>Premolded Expansion Joint Material</u> shall be provided where shown on the Drawings or directed by the Engineer. This non-extruding compressible joint material shall conform to the requirements of "Preformed Expansion Joint Fillers for Concrete Paving and Structural Construction", ASTM D1751.

2.02 CONCRETE MIXES

Ready-mixed concrete shall conform to "Specifications for Ready-Mixed Concrete", ASTM C94.

- A. All concrete mixes shall produce a dense durable concrete. The minimum 28-day compressive strength of the concrete shall be:
- B. 3,000 psi thrust blocking, sidewalks, curbs and pipe encasement. 4,000 psi manhole bases and road pavement
- C. Water/cement ratio for the concrete shall not exceed a maximum as shown in Table 4.4 of the ACI Standard 318 latest edition, Building Code Requirements for Reinforced Concrete, when strength data from field experience or trial mixtures are not available. A workable concrete with minimum slump of 3 inches and a maximum slump of 5 inches shall be produced without exceeding the water/ cement ratio.

PART 3: EXECUTION

3.01 FORMWORK

- A. Build all forms mortar tight and of sufficient rigidity to prevent distortion due to the pressure of the concrete and other loads incidental to the construction operations. Construct and maintain forms so as to prevent warping and the opening of joints.
- B. The forms shall be substantial and unyielding. Design the forms so that the finished concrete conforms to the proper dimensions and contours. Design the forms to take into account the effect of the vibration of concrete during placement.

3.02 PLACING REINFORCING STEEL

- A. Place all steel reinforcement accurately in the positions shown on the plans. Secure the steel reinforcements firmly in place during the placing and setting of concrete. When placed in the work, it shall be free from dirt, detrimental rust, loose scale, paint, oil or other foreign material. When spacing between crossing tie bars is one foot more, tie all bars at all intersections. When spacing is less than one foot in each direction tie alternate intersections of bars.
- B. Maintain distances from the forms by means of stays, blocks, ties, hangers or other approved supports. Continuous high chairs will not be permitted. Furnish all reinforcement in full lengths as indicated on the plans. Splicing of bars will not be permitted without the approval of the Engineer, except where shown on the plans. Stagger splices as far apart as possible. Unless otherwise shown on the plans, bars shall be lapped 36 diameters to make the splice.
- C. Lap welded wire mesh at least $1\frac{1}{2}$ meshes plus end extension of wires but not less than twelve (12) inches in structural slabs. Lap welded wire mesh at least $\frac{1}{2}$

mesh plus end extension of wires but not less than six (6) inches in slabs on the ground.

3.03 CONVEYING AND PLACING CONCRETE

- A. Convey concrete from the mixer to the forms as rapidly as practical by approved methods which will prevent segregation and loss of ingredients.
- B. Clean formwork of dirt and construction debris, drain water, and remove snow and ice. After the forms have been inspected, deposit the concrete in approximately horizontal layers to avoid flowing along the forms. Place all concrete in the dry free from standing water. Deposit all concrete continuously or in layers of a thickness such that no concrete will be deposited on concrete which has hardened sufficiently to cause the formation of seams and planes of weakness within the sections. Place the concrete to create a monolithic structure the component parts of which are securely bonded together. Compact the concrete during placement by suitable means. Work the concrete around the reinforcement and embedded fixtures and into corners and angles of forms, taking care to avoid overworking which may result in segregation.
- C. Do not drop concrete into forms from a height greater than 5 feet. Use a spout to deposit concrete from a greater height; or, provide openings in the forms limit the height of drop. Obtain the approval of the Engineer before using any other method of placing concrete from a height greater than 5 feet.
- D. Direct concrete through chutes to prevent it from striking reinforcement or sides of the form above the level of placement. Avoid segregation and coating of the surfaces with paste which may dry before concrete reaches its level.
- E. Submit a concrete mix design to the Engineer for approval prior to placing any concrete by pumping.

3.04 THRUST BLOCKING

- A. See the thrust blocking details. Notify the Engineer whenever field conditions are noted which are more restrictive than the thrust block design data included on Standard Detail 0201-0601-SD6.
- B. Construct blocking against the vertical face of undisturbed earth or sheeting left in place. Prevent the concrete from enclosing more than half the circumference of the pipe unless it is a straddle block. Keep the concrete away from joints or bolts in the piping.
- C. If thrust blocks are employed, place thrust blocking for hydrants to allow the hydrant to drain.

3.05 PLACING CONCRETE IN COLD WEATHER

A. Follow the provisions of ACI 306, ACI 308, and Paragraph 3.8 when the ambient temperature is less than 40°F at time of placement or expected to be less than 40°F during the curing period.

- B. Control concrete setting time with the use of accelerating admixtures as required to facilitate placing and finishing operations. Do not use calcium chloride in excess of 2% by weight in the concrete free of steel reinforcement. Where steel reinforcement is employed and concrete with calcium chloride is permitted, contractor must use galvanized or coated steel satisfactory to the Engineer.
- C. Exposed subgrade, formwork and reinforcing shall be warmer than 33°F prior to placement of concrete.
- D. The temperature of the concrete during placing shall be between 55°F and 75°F. Maintain the temperature of the concrete between 55°F and 75°F for a minimum of 5 days by providing insulating blankets, heated enclosures, or other methods of thermal protection. Provide a means of maintaining atmospheric moisture when dry heat is used. Provide proper curing for a minimum of days or as approved by the Engineer.
- E. In case of low air temperatures (below 40°F), submit a plan to comply with this section. The Engineer may, at their discretion, raise the minimum limiting temperatures for water, aggregates, and mixed concrete when temperatures drop below 40°F.
- F. Protect all earth supported concrete from damage due to frost heave.

END OF SECTION

PRECAST CONCRETE MANHOLE

PART 1: GENERAL

1.01 SCOPE

Furnish all labor, materials, tools, and equipment necessary to do all work required to install manholes as indicated on the Drawings and as specified in this Specification Section.

1.02 SUBMITTALS

Submit shop drawings or manufacturer's literature to the Engineer for approval.

PART 2: PRODUCTS

2.01 MANHOLE SECTIONS

Manhole riser sections shall be designed, manufactured, tested, finished and marked in accordance with the Drawings and ASTM C478, "Precast Reinforced Concrete Manhole Sections".

2.02 BRICK

Brick used to bring manhole to grade shall comply with ASTM C62, Grade SW.

2.03 LADDER

For heavy duty traffic manholes specified in section 2.04, ladder rungs shall be provided in accordance with OSHA regulations. Rungs shall have a minimum diameter of 1-inch and 10-inch clear tread width and be of the drop front design. Rungs shall be polypropylene coated ½-inch grade 60 deformed rebar by Lane International Corporation, M.A. Industries, or approve equal.

2.04 FRAME AND COVER

For areas of heavy-duty traffic or as noted on plans (flush mount), manhole frame with vented lid shall be Neenah Foundry Company's R-1752 Series Heavy Duty (36 inches round). For non-traffic areas with limited traffic (flush mount) Bilco's J-4H2O Series Floor, Vault and Sidewalk Door shall be used (36 inches square). For areas of non-traffic (mounted 8-12 inches above the surrounding surface). Bilco's J-4H2O Series Floor, Vault and Sidewalk Door shall be used (36 inches square).

PART 3: EXECUTION

3.01 HANDLING

Lift and move all precast manhole components using suitable lifting slings and plugs that will not damage the precast manhole lip.

Thoroughly repair all damage to precast sections in the presence of the Engineer. Repair and patch minor breaks by chipping and scarifying the defective area before applying grout. All sufficient curing time before the precast sections are put together. Form and key concrete cast-in-place bases specially to accommodate the bottom precast section.

3.02 INSTALLATION

Unless otherwise noted in the drawings, rest and support manhole bases uniformly on a 6-inch mat of compacted crushed stone or gravel placed over a base of sound, level, undisturbed earth.

Before placing concrete base, set the downstream and upstream pipes to proper grade so the pipe ends will be flush with the inside of the manhole.

Set the pipes securely in the opening of the precast sections of manholes and grout at the correct line and grade. There shall be at least a one-half inch clearance between the outside of the pipe and the manhole opening to insure proper grouting. Clean the pipe and base thoroughly before the grout is applied.

Bring the top of all precast manholes to proper grade for receiving manhole frames. If proposed grade is to be flush to existing grade, the top of the manhole ring and cover shall be within 1 inch of surrounding grade. If proposed grade is to be above existing grade, the top of the manhole ring and cover shall be between 8-12 inches of surrounding grade unless otherwise noted on the plans.

END OF SECTION

PIPING - GENERAL PROVISIONS

PART 1: GENERAL

1.01 DRAWINGS

A. Dimensions shown on Contract Drawings are approximate only. Verify all piping geometry in the field and to ensure proper alignment and fit of all piping consistent with the intent of the Contract Drawings. Submit field layout drawings as required for approval.

1.02 DAMAGE PREVENTION ACT

A. In addition to the Laws and Regulations included by reference in the General Conditions, Contractor shall comply and abide regulations by the local state UNDERGROUND FACILITIES DAMAGE PREVENTION AUTHORITY.

PART 2: PRODUCTS

2.01 CONTRACTOR'S RESPONSIBILITY FOR MATERIAL

- A. Examine all material carefully for defects. Do not install material which is known or thought to be defective.
- B. The Engineer reserves the right to inspect all material and to reject all defective material shipped to the job site or stored on the site. Failure of the Engineer to detect damaged material shall not relieve the Contractor from his total responsibility for the completed work if it leaks or breaks after installation.
- C. Lay all defective material aside for final inspection by the Engineer. The Engineer will determine if corrective repairs may be made, or if the material is rejected. The Engineer shall determine the extent of the repairs.
- D. Classify defective pipe prior to Engineer's inspection as follows:
 - 1. Damage to interior and/or exterior paint seal coatings.
 - 2. Damage to interior cement-mortar or epoxy lining.
 - 3. Insufficient interior cement-mortar lining or epoxy thickness.
 - 4. Excessive pitting of pipe.
 - 5. Poor quality exterior paint seal coat.
 - 6. Pipe out of round.
 - 7. Pipe barrel area damaged to a point where pipe class thickness is reduced (all pipe).
 - 8. Denting or gouges in plain end of pipe (all pipe).
 - 9. Excessive slag on pipe affecting gasket seal (DI).
 - 10. Any visible cracks, holes.
 - 11. Embedded foreign materials.
 - 12. Non-uniform color, density and other physical properties along the length of the pipe.

- E. The Contractor shall be responsible for all material, equipment, fixtures, and devices furnished. These materials, equipment, fixtures and devices shall comply with the requirements and standards of all Federal, State, and local laws, ordinances, codes, rules, and regulations governing safety and health.
- F. Take full responsibility for the storage and handling of all material furnished until the material is incorporated in the completed project and accepted by the Engineer. Contractor shall be solely responsible for the safe storage of all material furnished to or by him until incorporated in the completed project and accepted by the Engineer.
- G. Load and unload pipe, fittings, valves, hydrants and accessories by lifting with hoists or skidding to avoid shock or damage. Do not drop these materials. Pipe handled on skidways shall not be skidded or rolled against another pipe. Handle this material in accordance with AWWA C600, C605 or C906 whichever is applicable.
- H. Drain and store fittings and valves prior to installation in such a manner as to protect them from damage due to freezing of trapped water. Drain, store, and protect fittings and valves in accordance with Specification Section 01600.

2.02 PETROLATUM TAPE COATING

- A. The tape coating shall be a cold applied, saturant tape made from either petrolatum or petroleum wax with a noncellulosic synthetic fiber fabric. The fabric shall be encapsulated and coated on both sides with the petrolatum or petroleum wax. The thickness of the tape shall be no less than 40 mil. The petrolatum or petroleum wax shall be at least 50% of the product by weight.
- B. The tape coating shall be supplied in sheets, pads or rolls. Pads and sheets shall be sized to fit the area that is to be covered, allowing for an overlap per AWWA Standards.

2.03 RUBBERIZED-BITUMEN BASED SPRAY-ON UNDERCOATING

A. Subject to approval by the Engineer, an alternative corrosion protection for exposed buried metal is an aerosol applied rubberized coating. The material shall be rapid dry and specifically designed for corrosion protection. 3M Rubberized Underseal Undercoating 08883 or any equivalent rubberized-bitumen based spray-on undercoating may be used. Follow manufacturer's recommendations for storage and application.

PART 3: EXECUTION

3.01 INSTALLATION - GENERAL REQUIREMENTS

A. Lay and maintain all pipe to the required lines and depths. Install fittings, valves and hydrants in strict accordance with the Specifications at the required locations with joints centered, spigots home, and all valve and hydrant stems plumb. Do not deviate from the required alignment, depth or grade without the written consent of the Engineer.

- B. Buried steel lugs, rods, brackets, and flanged joint nuts and bolts are not permitted unless specifically shown on the drawings or approved in writing by the Engineer. Cover any and all buried steel lugs, rods, brackets, and flanged joint nuts and bolts with approved coating in accordance with AWWA C217 prior to backfilling. Encase the same in polyethylene encased if the specifications require polyethylene encasement of the pipe.
- C. Lay all pipe to the depth specified. Measure the depth from the final surface grade to the top of the pipe barrel. The minimum pipe cover shall be 3.5 feet (42 inches) provided over the top of the pipe to prevent freezing as shown on the Drawings or as specified in the Specifications Special Conditions.
- D. Dead ends shall be minimized by looping mains wherever possible. All dead ends shall be provided with a fire hydrant, flushing hydrant or blow off valve in accordance with Hartford County waterworks regulations.
- E. Do not lay pipe in a wet trench, on subgrade containing frost, or when trench conditions are unsuitable for such work. If all efforts fail to obtain a stable dry trench bottom and the Engineer determines that the trench bottom is unsuitable for such work, the Engineer will order the kind of stabilization to be constructed, in writing. In all cases, water levels must be at least 6 inches below the bottom of the pipe. See Section 02020, Dewatering.
- F. Thoroughly clean the pipes and fittings before they are installed. Keep these materials clean until the acceptance of the completed work. Lay pipe with the bell ends facing in the direction of laying, unless otherwise shown on the Drawings, or directed by the Engineer. Exercise care to ensure that each length abuts the next in such a manner that no shoulder or unevenness of any kind occurs in the pipeline.
- G. Do not wedge or block the pipe during laying unless by written order of the Engineer.
- H. Before joints are made, bed each section of pipe the full length of the barrel, at the required grade, and at the invert matching the previously laid pipe. Dig bell holes sufficiently large to permit proper joint making. Do not bring succeeding pipe into position until the preceding length is embedded and secure in place.
- I. Take up and relay pipe that is out of alignment or grade, or pipe having disturbed joints after laying. Take up, such in-place pipe sections found to be defective and replace them with new pipe. Take up, relaying, and replacement will be at the Contractor's expense.
- J. Place enough backfill over the center sections of the pipe to prevent floating. Take all other necessary precautions to prevent the floating of the pipeline by the accumulation of water in the trench, or the collapse of the pipeline from any cause. Place enough backfill over the center sections of the pipe to prevent floating. Should floating or collapse occur, restoration will be at the Contractor's expense.

- K. Bedding materials and concrete work for the pipe bedding and thrust restraint shall be as specified in Divisions 2, 3, and 15 as well as in Standard Detail drawings.
- L. Prevent foreign material from entering the pipe while it is being placed. Do not place debris, tools, clothing, or other materials in the pipe during laying operations. Close all openings in the pipeline with watertight plugs when pipe laying is stopped at the close of the day's work, or for other reasons such as rest breaks or meal periods.
- M. Only cut pipe with equipment specifically designed for cutting pipe such as an abrasive wheel, a rotary wheel cutter, a guillotine pipe saw, or a milling wheel saw. Do not use chisels or hand saws. Grind cut ends and rough edges smooth. Bevel the cut end slightly for push-on connections as per manufacturer recommendations.
- N. In distributing material at the site of the Work, unload each piece opposite or near the place where it is to be laid in the trench. If the pipe is to be strung out, do so in a straight line or in a line conforming to the curvature of the street. Block each length of pipe adequately to prevent movement. Block stockpiled pipe adequately to prevent movement. Do not place pipe, material, or any other object on private property, obstructing walkways or driveways, or in any manner that interferes with the normal flow of traffic.
- O. Exercise special care to avoid damage to the bells, spigots or flanged ends of pipe during handling, temporary storage, and construction. Replace damaged pipe that cannot be repaired to the Engineer's satisfaction, at the Contractor's expense.
- P. Remove all existing pipe, fittings, valves, pipe supports, blocking, and all other items necessary to provide space for making connections to existing pipe and installing all piping required under this Contract.
- Q. Maintain the minimum required distance between the water line and other utility lines in strict accordance with all Federal, State, and local requirements and all right-of-way limitations.
- R. Provide and install polyethylene encasement for ductile iron pipe as required by the Drawing or Specification Special Conditions. See Specification Section 15130 or 15131, as applicable.
- S. The maximum allowable deflection at the joints for push-on joint pipe shall be the lesser of manufacturer's recommendations or as described in the DIPRA Guideline, *Ductile Iron Pipe Joints and Their Uses*, as follows:

Size of	Deflection	Maximum Deflection		
<u>Pipe</u>	Angle	<u>(18-ft. Length)</u>	<u>(20-ft. Length)</u>	
3"-12"	5 degrees	19"	21"	
14"-42"	3 degrees	11"	12"	
48"-64"	3 degrees	N/A	12"	

- T. Use short lengths of pipe (minimum length 3 feet, no more than three short sections), when approved by the Engineer, to make curves that cannot be made with full length sections of pipe without exceeding the allowable deflection. Making these curves will be at no additional cost to the Owner.
- U. Furnish air relief valve assemblies in accordance with detail drawings provided or as specified in the specification Special Conditions section. Engineer will provide standard detail for additional air release valve assemblies. Any deviation from the standard detail proposed by contractor must be approved in advance.
- V. Exercise particular care so that no high points are established where air can accumulate. Install an air release valve and manhole, as extra Work to the Contract, when the Engineer determines that unforeseen field conditions necessitate a change in the pipe profile that requires the installation of an air release valve and manhole. If the Contractor requests a change in the pipe profile solely for ease of construction, and the requested change requires the installation of an air release valve and manhole as determined by the Engineer, the cost of furnishing and installing the air release valve and manhole will be at the expense of the Contractor.

3.02 CONSTRUCTION METHODS TO AVOID CONTAMINATION

- A. Heavy particulates generally contain bacteria and prevent even very high chlorine concentrations from contacting and killing such organisms. It is essential that the procedures of this Specification Section be observed to assure that a water main and its appurtenances are thoroughly clean for the final disinfection by chlorination.
- B. Take precautions to protect the interior of pipes, fittings, and valves against contamination. String pipe delivered for construction so as to keep foreign material out of the pipe. Close all openings in the pipeline with watertight plugs when pipe laying is stopped at the close of the day's work or for other reasons, such as rest breaks or meal periods. Use rodent-proof plugs approved by Engineer, where it is determined that watertight plugs are not practical and where thorough cleaning will be performed.
- C. Delay in placement of delivered pipe invites contamination. The more closely the rate of delivery is correlated to the rate of pipe laying, the lower the likelihood of contamination. Complete the joints of all pipe in the trench before stopping work. If water accumulates in the trench, keep the plugs in place until the trench is dry.
- D. When encountering conditions on pre-existing pipe that requires packing, employ yarning or packing material made of molded or tubular rubber rings, or rope of treated paper or other approved materials. Do not use materials such as jute, asbestos, or hemp. Handle packing material in a manner that avoids contamination.
- E. Do not use contaminated material or any material capable of supporting prolific growth of microorganisms for sealing joints. Handle sealing material or gaskets in a manner that avoids contamination. The lubricant used in the installation of

sealing gaskets shall be suitable for use in potable water. Deliver the lubricant to the job in closed containers and keep it clean.

- F. If dirt enters the pipe, and in the opinion of the Engineer the dirt will not be removed by the flushing operation, clean the interior of the pipe by mechanical means, then swab with a 1% hypochlorite disinfecting solution. Clean using a pig, swab, or "go-devil" only when the Engineer has specified such and has determined that such operation will not force mud or debris into pipe joint spaces.
- G. If the main is flooded during construction, the flooded section must be isolated from the remainder of the installation as soon as practical. Submit a plan to the Engineer on correcting the condition and do not proceed until authorized by the Engineer. Replace or fully clean and disinfect the affected pipe at no additional cost to the Owner.

3.03 VALVE INSTALLATION

- A. Prior to installation, inspect valves for direction of opening, freedom of operation, tightness of pressure containing bolting, cleanliness of valve ports and especially of seating surfaces, handling damage, and cracks. Correct defective valves or hold for inspection by the Engineer.
- B. Set and join to the pipe in the manner specified in Specification Section 3.01. Provide valves with adequate support, such as crushed stone and concrete pads, so that the pipe will not be required to support the weight of the valve. Set truly vertical. After field installation of the valve all exposed ferrous restraint materials and external bolts except the operating nut shall receive a layer of petrolatum tape coating or, where approved, rubberized-bitumen based spray-on undercoating applied before backfill. If polyethylene is applied to the pipe, the entire valve shall be encased in polyethylene encasement prior to backfill. The polyethylene encasement shall be installed up to the operating nut leaving the operating nut exposed and free to be operated.
- C. Provide a valve box for each valve. Set the top of the valve box neatly to existing grade, unless directed otherwise by the Engineer. Do not install in a way that allows the transfer shock or stress to the valve. Center and plumb the box over the wrench nut of the valve. Do not use valves to bring misaligned pipe into alignment during installation. Support pipe in such manner as to prevent stress on the valve. See Standard Detail 0201-0601-SD59 for a typical valve box installation detail.
- D. Provide valve marking posts, when authorized by the Owner, at locations designated by the Engineer and in accordance with detail drawings (included at the end of this Specification Section). Payment will be made per post in accordance with supplemental unit price schedule.

3.04 THRUST RESTRAINT

- A. Provide all plugs, caps, tees, and bends (both horizontal and vertical) with concrete thrust blocking and/or restrained joint pipe as represented on the Drawings or specified in the Specification Special Conditions.
- B. Place concrete thrust blocking between undisturbed solid ground and the fitting to be anchored. Install the concrete thrust blocking in accordance with Specification Section 03300 and standard details provided. Locate the thrust blocking to contain the resultant thrust force while keeping the pipe and fitting joints accessible for repair, unless otherwise shown or directed.
- C. Provide temporary thrust restraint at temporary caps and plugs. Submit details of temporary restraint to the Engineer for approval.
- D. At connections with existing water mains where there is a limit on the time the water main may be removed from service, use metal harnesses of anchor clamps, tie rods and straps; mechanical joints utilizing set-screw retainer glands; or restrained push-on joints as permitted by Engineer. No restraining system can be installed without the approval of the Engineer. Submit details of the proposed installation to the Engineer for approval. For pipe up to 12 inches in size, use a minimum of two 3/4-inch tie rods. If approved for use, install retainer glands in accordance with the manufacturer's instructions. Material for metal harnessing and tie-rods shall be ASTM A36 or A307, as a minimum requirement.
- E. Protection of Metal Harnessing: Protect ties rods, clamps and other metal components against corrosion by hand application of petrolatum tape and by encasement of the entire assembly with 12 mil thick loose polyethylene film in accordance with AWWA C105. Apply tape on all exposed tie rods prior to installing polyethylene.

3.05 TYPICAL INSTALLATION DETAILS

A. The list of Standard Details is included in the table of contents of the Contract Documents.

END OF SECTION

DISINFECTING PIPELINES

PART 1: GENERAL

1.01 SCOPE OF WORK

Flush and disinfect all pipelines installed under this Contract if indicated in the summary of work. This would include furnishing the necessary labor, tools, transportation, and other equipment for the operation of valves, hydrants, and blowoffs during the chlorination. Install, and if directed remove, all chlorination taps required for disinfection. The cost of this work shall be included in the bid item for pipe installation. The disinfection will be performed under the supervision of Owner.

1.02 WORK BY Developer

Developer shall provide/furnish all chlorine and chlorination equipment, and furnish water for testing, flushing and disinfecting pipelines.

American Water will collect the sample and perform bacteriological testing. If approved by American Water engineer, Developer may allow to collect the sample and perform bacteriological testing

1.03 PROTECTION

Chlorine disinfection and dechlorination shall be under the direct supervision of someone familiar with the physiological, chemical, and physical properties of the form of chlorine used. They shall be trained and equipped to handle any emergency that may arise. All personnel involved shall observe appropriate safety practices to protect working personnel and the public.

The forwards of AWWA Standards B300 and B301 contain information and additional reference material regarding the safe handling of hypochlorites and liquid chlorine. The Contractor shall familiarize himself with this information prior to performing any disinfection work.

1.04 RELATED WORK

Observe the precautions described in Specification Section 15000 to avoid contamination during installation of the pipeline.

1.05 REFERENCES

Refer to current AWWA Standard for Disinfecting Water Mains C651-14 and Supplementary Conditions Appendix I – Disposal of Chlorinated Waters. All Work shall be in accordance with the latest State and City/County Code.

PART 2: PRODUCTS

2.01 MATERIALS AND EQUIPMENT

- A. Furnish liquid chlorine and injection equipment and/or calcium hypochlorite (HTH) as needed to disinfect all pipelines and appurtenances.
- B. Liquid chlorine contains 100% available chlorine and is packaged in steel containers, usually of 100 lb, 150 lb, or 1 ton net chlorine weight. Liquid chlorine is to be furnished in accordance with AWWA B301.
- C. Calcium hypochlorite is available in granular form or in approximately 5-gram tablets, and contains approximately 65% available chlorine by weight and is employed in calculations used in this specification. The material should be stored in a cool, dry, and dark environment to minimize its deterioration. Do not use calcium hypochlorite intend for swimming pool disinfection, as this material (containing trichloroisocyanuric acid) has been sequestered and is extremely difficult to eliminate from the pipe after the desired contact time had been achieved.
- D. Calcium hypochlorite must conform to AWWA B300.

PART 3: EXECUTION

3.01 PREPARATION

All pipelines shall be pressure and leak tested, flushed, and cleaned of debris and dirt prior to application of the disinfectant. Flushing shall continue until the volume in the newly installed main has turned over at least one time unless the Engineer determines that conditions do not permit the required volume to be safely discharged to waste.

3.02 APPLICATION OF DISINFECTANT

Methods to be used for disinfection are those detailed in ANSI/AWWA C651-14 Disinfecting Water Mains.

3.03 WATER MAINS

Three (3) methods of chlorination are described below. The third method, using tablets of hypochlorite, is only permitted by expressed approval of the Engineer and under no circumstance allowed for projects of 2000 feet or more. Otherwise, information in the forward of AWWA Standard C651 will be helpful in determining the best method to be used.

- A. <u>Continuous Feed Method</u>
 - 1. <u>Setup</u>

The continuous feed method consists of completely filling the main to remove all air pockets, flushing the completed main to remove particulates, and then refilling the main with chlorinated potable water. The potable water shall be chlorinated, so that after a 24-hour holding period in the main, there will be a free chlorine residual of not less than 10 mg/L in collected samples.

Chlorine can be applied in advance of preliminary flushing by swabbing joints with bleach or placing hypochlorite granules in the pipe in areas where contamination is suspected. In any such case, the contractor shall make sure and take appropriate action to make sure that the flushed water is dechlorinated.

2. Preliminary Flushing

Prior to being chlorinated, fill the main to eliminate air pockets and flush to remove particulates. The flushing velocity in the main shall be not less than 2.5 fps unless the Engineer determines that conditions do not permit the required flow to be discharged to waste. Table 1 shows the rates of flow required to produce a velocity of 2.5 fps in pipes of various sizes.

NOTE: Flushing is no substitute for preventive measures during construction. Certain contaminants such as caked deposits resist flushing at any feasible velocity.

		TABL	<u>E 1</u>		
	Required Flow a	nd Open	ings to	<u>Flush F</u>	<u>Pipelines</u>
	<u>(</u> 40 psi Resid	ual Press	ure in \	Nater N	<u>Main)*</u>
Pipe Diameter	Flow required to produce 2.5 fps velocity in main	1	Size of <u>(inche</u> 1-1/2	Tap <u>es)</u> 2	Number of 2-1/2 in. Hvdrant
<u>(inches)</u>	<u>(qpm)</u>	Numbe	er of tap	s on P	ipe [†] Outlets to Use
4	100	1	-	-	1
6	200	-	1	-	1
8	400	-	2	1	1
10	600	-	3	2	1
12	900	-	-	2	2
16	1600	-	-	4	2

*With a 40-psi pressure in the main with the hydrant flowing to atmosphere, a $2\frac{1}{2}$ -inch hydrant outlet will discharge approximately 1,000 gpm and a $4\frac{1}{2}$ -inch hydrant outlet will discharge approximately 2,500 gpm.

† Number of taps on pipe based on discharging through 5 feet of galvanized iron pipe with one 90-degree elbow.

In mains of 24-inches or larger diameter, an acceptable alternative to flushing is to broom-sweep the main, carefully removing all sweepings prior to chlorinating the main.

- 3. Chlorinating the Main
 - a. Flow water from the existing distribution system or other approved source of supply at a constant, measured rate into the newly laid water main. In the absence of a meter, approximate the rate by placing a pitot gauge in the discharge or measuring the time to fill a container of known volume.
 - b. At a point not more than 10 feet downstream from the beginning of the new main, dose the water entering the new main with chlorine fed at a constant rate such that the water will have not less than 25 mg/L free chlorine. Measure the chlorine concentration at regular intervals to ensure that this concentration is provided. Measure chlorine in accordance with the procedures described in the current edition of the AWWA Manual M12 or of *Standard Methods for the Examination of Water and Wastewater*.
 - c. Table 2 gives the amount of chlorine required for each 100 feet of pipe of various diameters. Solutions of 1% chlorine may be prepared with calcium hypochlorite and the table indicates the appropriate amount of the 65% calcium hypochlorite. If using other concentrations of calcium hypochlorite, a properly adjusted weight must be used. A 1% chlorine solution requires 1 pound of calcium hypochlorite in 8 gallons of water.

Pipe Diameter	100 Percent	65 Percent	1 Percent
inches	lbs	lbs	gallons
4	0.013	0.020	0.16
6	0.030	0.046	0.36
8	0.054	0.083	0.65
10	0.085	0.131	1.02
12	0.120	0.185	1.44
16	0.217	0.334	2.60

TABLE 2
Chlorine and Hypochlorite Required to Produce 25 mg/L
Concentration in 100 feet of Pipe by Diameter

d. During the application of chlorine, position valves so that the strong chlorine solution in the main being treated will not flow into water mains in active service. Do not stop the chlorine application until the entire main is filled with heavily chlorinated water. Keep the chlorinated water in the main for at least 24 hours. During this time, operate all valves and hydrants in the section treated in order to disinfect the appurtenances. At the end of this 24-hour period, the treated water in all portions of the main shall have a residual of not less than 10 mg/L free chlorine.

- e. Hypochlorite solution may be applied to the water main with a gasoline or electrically powered chemical feed pump designed for feeding chlorine solutions. Feed lines shall be of such material and strength as to safely withstand the corrosion caused by the concentrated chlorine solutions and the maximum pressures that may be created by the pumps. Check all connections shall for tightness before the solution is applied to the main.
- f. If gaseous chlorine in solution is permitted by the Engineer and proposed by the contractor, the preferred equipment for the gas application employs a feed vacuum-operated chlorinator to mix the chlorine gas, in combination with a booster pump for injecting the chlorine gas solution water into the main to be disinfected. Direct feed chlorinators cannot be used. (A direct feed chlorinator is one which operates solely from the pressure in the chlorine cylinder.)

B. Slug Method

- 1. <u>Setup</u>
 - a. The slug method consists of placing calcium hypochlorite granules in the main during construction; completely filling the main to eliminate all air pockets, flushing the main to remove particulates, and slowly flowing a slug of water containing 100 mg/L of free chlorine through the main so that all parts of the main and its appurtenances will be exposed to the highly chlorinated water for a period of not less than 3 hours.
- 2. Chlorinating the Main
 - a. At the option of the Owner, place calcium hypochlorite granules in the main during construction. The purpose of this procedure is to provide a strong chlorine concentration in the first flow of flushing water especially to fill annular spaces in pipe joints. Flush the main to eliminate air and remove particulates to include management of dechlorination and discharged water.
 - b. At a point not more than 10 feet downstream from the beginning of the new main, dose the water entering the new main with chlorine fed at a constant rate such that the water will have not less than 100 mg/L free chlorine. Measure the chlorine concentration at regular intervals to ensure that this concentration is provided. Measure chlorine in accordance with the procedures described in the current edition of the AWWA Manual M12 or of *Standard Methods for the Examination of Water and Wastewater*. The chlorine shall be applied continuously and for a sufficient period to develop a solid column or "slug" of chlorinated water that will, as it moves through the main, expose all interior surfaces to a concentration of approximately 100 mg/L for at least 3 hours.
 - c. The free chlorine residual shall be measured in the slug as it moves through the main. If at any time it drops below 50 mg/L, stop the flow, relocate the chlorination equipment to the head of the slug,

and as flow is resumed, apply chlorine to restore the free chlorine in the slug to not less than 100 mg/L.

d. As the chlorinated water flows past fittings and valves, operate related valves and hydrants so as to disinfect appurtenances and pipe branches.

C. Tablet Method

- 1. <u>Setup</u>
 - a. The tablet method consists of adhering calcium tablets in the water main as it is being installed and then filling the main with potable water when installation is completed. This method may be used only if the pipes and appurtenances are kept clean and dry during construction and with permission by the Engineer for short main installations.
- 2. Chlorinating the Main
 - a. Placing of calcium hypochlorite tablets. During construction, 5-g calcium hypochlorite tablets shall be placed in each section of pipe. Also, one such tablet shall be placed in each hydrant, hydrant branch, and other appurtenance. The number of 5-g tablets required for each pipe section shall be 0.0012 d²L rounded to the next higher integer, where *d* is the inside pipe diameter, in inches, and L is the length of the pipe section, in feet. Table 1 shows the number of tablets required for commonly used sizes of pipe. The tablets shall be attached by a food-grade NSF approved adhesive. There shall be no adhesive on the tablet except on the broadside attached to the surface of the pipe and no adhesive applied or spilled on the pipe surface. Excess adhesive must be removed immediately using mechanical means or an NSF approved adhesive solvent. Attach all the tablets inside and at the top of the main, with approximately equal numbers of tablets at each end of a given pipe length. If the tablets are attached before the pipe section is placed in the trench, their position shall be marked on the section so it can be readily determined that the pipe is installed with the tablets at the top.

Pipe Diameter		Length of Pipe Section, ft (m)				
		13(4.0) or less	18(5.5)	20(6.1)	30(9.1)	40(12.2)
in.	(<i>mm</i>)	Number of 5-g Calcium Hypochlorite Tablets				
6	(150)	1	1	1	2	2
8	(200)	1	2	2	3	4
12	(300)	3	4	4	6	7
16	(400)	4	6	7	10	13
b. Filling and contact. When installation has been completed, the main shall be filled with water at a rate such that water within the main will flow at a velocity no greater than 1 ft/s (0.3 m/s). Precautions shall be taken to ensure that air pockets are eliminated. This water shall remain in the pipe for at least 24 hours. If the water temperature is less than 41°F (5°C), the water shall remain in the pipe for at least 48 hours.

3.04 DISPOSAL OF HEAVILY CHLORINATED WATER

- A. Do not keep heavily chlorinated water in contact with pipe for more than 48 hours after the applicable retention period. In order to prevent damage to the pipe lining or corrosion damage to the pipe itself, flush the heavily chlorinated water from the main fittings, valves, and branches until chlorine measurements show that the concentration in the water leaving the main is no higher than that generally prevailing in the system or is acceptable for domestic use. Take all steps necessary to dechlorinate water where required per Section 3.04B and 3.04C below and Appendix I Disposal of Chlorinated Waters of the Supplementary Conditions. Contact the local sewer department to arrange for disposal of the heavily chlorinated water to the sanitary sewer if applicable.
- B. Neutralize the chlorine residual of the water being disposed of by treating with one of the chemicals listed in Table 3. Select an alternative disposal site if a sanitary sewer system is unavailable for disposal of the chlorinated water.
- C. The proposed alternative disposal site shall be inspected and approved of by the Engineer. Apply a reducing agent to the chlorinated water to be wasted to completely neutralize the chlorine residual remaining in the water. (See Table 3 for neutralizing chemicals. Do not overdose neutralizing chemicals as this may result in adverse environmental impacts. Only dose the amount required to neutralize the amount of chlorine present). Contact federal, state and local regulatory agencies, where necessary, to determine special provisions for the disposal of heavily chlorinated water.

Table 3						
Pound	ds of chen	nicals require	<u>ed to neutr</u>	<u>alize various</u>		
Residual	<u>chlorine c</u>	oncentratior	ns in 100,00	00 gallons of water		
Residual Chlorine	Sulfur	Sodium	Sodium	Sodium	Ascorbic	
Concentration	Dioxide	Bisulfite	Sulfite	Thiosulfate	Acid	
<u>mg/L</u>	<u>(SO₂)</u>	<u>(NaHSO₃)</u>	<u>(Na₂SO₃)</u>	$\underline{(Na_2S_2O_3\cdot 5H_2O)}$	$(C_6O_8H_6)$	
1	0.8	1.2	1.4	1.2	2.1	
2	1.7	2.5	2.9	2.4	4.2	
10	8.3	12.5	14.6	12.0	20.9	
50	41.7	62.6	73.0	60.0	104.0	

- D. Test for chlorine residual throughout the disposal process to be sure that the chlorine is neutralized.
- E. Submit a plan of disposal of flushed water to the Engineer for approval.

3.05 BACTERIOLOGICAL TESTING

- A. After disinfection and flushing at a minimum velocity of 3 ft/sec;
 - 1. Take an initial set of samples and then resample again after a minimum of 16 hours.
 - Or
 - 2. Allow the water to sit for a minimum of 16 hours without any water use. Then collect two sets of samples, without flushing the main, a minimum of 15 minutes apart while the sample taps are left running.

Two (2) consecutive sets of bacteriological samples shall be collected from the new main and must indicate no coliform contamination before the pipe can be put into service.

Collection of all samples from the new main shall conform to the procedures in accordance with the latest edition of AWWA C651-14. Chlorine residual shall be tested before sampling and must be 1 ppm or less before the sample is collected. At least one set of samples shall be collected at intervals of 1,200 feet (366 m) along the new water main, in addition to one set from the end of the line and at least one set from each branch greater than one pipe length. Refer to Section 5.1.1.3 of AWWA C651-14 for more stringent testing requirements if trench water has entered the new main.

- B. Samples shall be collected by a person knowledgeable in collecting samples for bacteriological sampling or arrange for the American Water to collect the sample. Coordinate with American Water and submit samples to the American Water for testing of bacteriological (chemical and physical) quality. Testing will be in accordance with <u>Standard Methods of the Examination of Water and Wastewater</u>. Samples shall show the absence of coliform organisms; and the presence of a chlorine residual. Samples shall also be tested for turbidity, pH, and standard heterotrophic plate count (HPC). HPC levels must be consistent with levels normally found in the distribution system to which the new main is connected.
- C. Bacteriological tests must show complete absence of coliforms and acceptable HPCs. If tests show the presence of coliform or unacceptable HPCs, perform additional flushing and disinfection of the pipeline until acceptable tests are obtained, all at no cost to the American Water.

3.06 RETESTING AND TESTING SOURCE WATER

- A. At the time of initial flushing the main to remove material and test for air pockets, Contractor may request the Owner to continue flushing until the desired chlorine residual is met at the discharge point. Notification must be provided in advance and the Contractor shall be prepared to test for chlorine at intervals of no more than five minutes as the water clears. This will provide the Contractor with some assurance that the source water is chlorinated.
- B. If the subsequent tests for bacteriological contamination conducted by the Contractor fail, the Contractor may request the Owner to continue flush from the source water into the new pipe system until a chlorine residual is found at the discharge point. Notification must be provided in advance and the Contractor shall

be prepared to test for chlorine at intervals of no more than five minutes as the water clears. The operation of all existing system valves shall be by the Owner at the Contractor expense and the discharge point must be opened prior to opening existing valves to avoid contamination. This will provide the Contractor with some assurance that the source water is chlorinated for subsequent tests.

CLEANING PIPELINES

PART 1: GENERAL

1.01 SCOPE OF WORK

Clean the pipelines installed under these Contract Documents using foam pigs, swabs, or "go-devils", as described herein, whenever normal flushing will not sufficiently remove dirt and debris that was introduced during construction.

1.02 GENERAL

Normal pipeline flushing is often inadequate to remove all the entrapped air, loose debris, and other objects that may have been left in the main during installation. In such cases, use polyurethane foam pigs and/or polyurethane hard foam swabs to remove all foreign matter from the pipeline (i.e. "pig" the pipeline). Clean the pipeline per the requirements of this Specification Section prior to testing and disinfecting the main.

1.03 RELATED WORK

Section 15000.3.02 – Construction Methods to Avoid Contamination Section 15020.3.01 – Preparation (see prior to disinfecting pipelines)

1.04 PROTECTION DURING FLUSHING AND CLEANING

Coordinate with Engineer and Owner before flushing to ensure that an adequate volume of flushing water is available, at sufficiently high pressure. Determine if the water can be disposed of safely. Notify the Owner, Engineer, and the following prior to flushing, or cleaning:

- A. Fire Department
- B. Other utilities, such as gas, electric and telephone companies, who may have underground facilities in the area.
- C. Customers who may be inconvenienced by reduced pressure or dirty water.

Coordinate with Owner to isolate the section to be flushed from the operating distribution system. Close valves slowly to prevent water hammer. Open the fire hydrant or blow-off valve slowly until the desired flow rate is obtained. When flushing from a dry barrel fire hydrant, use the gate valve upstream of the hydrant for throttling purposes. Open the hydrant valve fully to prevent water from escaping into the ground through the fire hydrant barrel drain.

Protect the work staff and the public during operation of hydrants and valves. Keep children away from the flow of flushing water. Where practical employ energy dissipators to help avoid damage to property and the flooding of streets. The safety considerations also apply to main cleaning. See General Conditions Article 6.

PART 2: PRODUCTS

2.01 MATERIALS AND EQUIPMENT

Furnish the foam cleaning plugs (swabs or pigs), labor, and equipment as needed to pig all pipelines. Furnish all materials required for the expulsion of air and other debris from pipelines. Do not use of pipe cleaning plugs which utilize Bristles, wire brushes, carbide abrasives, steel studs, or any other Type abrasive unless specifically approved by the Engineer. Consult a manufacturer of pipe cleaning plugs, such as Knapp Polly Pig (Houston, Texas), to determine the type and size of cleaning plug best suited for the application. Two types of plugs shall be considered and are described as follows:

A. <u>Swabs</u>

Swabs used for cleaning mains shall be made of polyurethane foam. This foam has a density of 1 to 2 pounds per cubic feet. Swabs shall be purchased from commercial manufacturers of swabs for pipes. Both soft and hard grade foam swabs are available. New mains are typically cleaned with hard foam swabs.

Use swabs cut into cubes and cylinders slightly larger than the size of the pipe to be cleaned. Cubes one inch larger in dimension than the nominal diameter of the pipe being cleaned have worked well for cleaning pipes up to 12-inches in diameter. For mains greater than 12-inches in diameter, the swab diameter must be considered individually for each operation. For new mains, swabs 3-inches larger than the pipe diameter have worked well. Swabs for the larger mains are usually 1-1/2 times the diameter in length.

B. <u>Pigs</u>

The other type of cleaning plug available is called a pig. Pigs, if used, shall be commercially manufactured for the specific purpose of cleaning pipes. They shall be made of polyurethane foam weighing 2 to 15 pounds per cubic feet. Pigs are bullet shaped and come in various grades of flexibility and roughness. Pigs are typically 1/4-inch to 1/2-inch larger in diameter than the pipe to be cleaned.

PART 3: EXECUTION

3.01 PLUG INSTALLATION AND REMOVAL

Furnish all equipment, material, and labor to satisfactorily expose cleaning wyes, or other entry or exit points. Remove cleaning wye covers, etc., as required by the Engineer to insert the plugs into the mains.

If approved by the Engineer, stripped fire hydrants, air valves and blow-offs may serve as entry and exit points for smaller sized mains. The Engineer will examine these appurtenances and the connecting laterals to ensure that adequate openings exist through which a plug may be launched.

If these appurtenances are used, a special launcher is required to ease the insertion and launching of the plug. If available, a pressurized water source such as a fire hydrant can

be used to launch the plug. If water from the system is not available nearby, use a water truck with pump.

If hydrants are used as entry and/or exit points, remove the internal mechanisms and plug the drains under the supervision of the Engineer. Insert the plug and replace the cap with a special flange with a 2-1/2-inch fitting. Connect the 2-1/2-inch fitting, with a pressure gauge and valve, to a pressurized water source. After closing the last valve isolating the section to be cleaned, open the hydrant supply valve. Propel the swab or pig into the main by opening the exit valve.

In mains greater than 8-inches, wyes shall be used at the entry and exit points. Fabricate the wye section one size larger than the main to ease the insertion and extraction of the plug. The use of wyes, as with the previously mentioned appurtenances, requires an outside source of pressurized water for launching. Cap the wye with a flange with a 2- to 6-inch fitting for connecting to the pressurized water source.

Many pigs are harder to insert into a pipe since they are less flexible than swabs. Other methods acceptable to insert pigs include:

- A. winching with a double sling,
- B. winching with a rope attached to the pig,
- C. compression with a banding machine prior to insertion, and
- D. the use of a specially designed tapered steel pipe which is removed after use.

During swab or pig installation, leave as much water as possible in the main to be cleaned. The water suspends the material being removed from the pipe and minimizes the chance of the material forming a solid plug. Water in the pipe also keeps the swab or pig from traveling through the pipe at excessive rates. If swabs or pigs travel too fast, they will remove less material and wear more rapidly.

At the exit point or blow-off, install a wye long enough to house the swab or pig. Attach temporary piping to the end cap to allow the drainage of the water.

Take precautions to prevent backflow of purged water into the main when the cleaning plug exits through a dead end main. This can be accomplished by installing mechanical joint bends and pipe joints to provide a riser out of the trench. Additional excavation of the trench may serve the same purpose.

3.02 PRE-CLEANING PROCEDURES

- A. Prepare a written cleaning plan for the Engineer's review.
- B. Suggested pre-cleaning procedures include:
 - 1. Identify mains to be cleaned on a map. Mark the location of the entry, water supply, exit points, any blow-offs to be used, valves to be closed, and the path of the swab or pig.

- 2. Under the Engineer's supervision and with Owner staff as required, inspect and operate all valves and hydrants to be used in the cleaning operation to ensure their correct operation and a tight shutdown.
- 3. Check location and type of hydrants, launch and exit location, and blowoffs to be used. Make blow-off tap connections, if necessary.
- 4. The Owner will notify customers served by the main to be cleaned that their water will be off for a specified period of time on the day of the cleaning.
- 5. The Owner will identify customers who may require temporary services during the main cleaning operation. The Contractor shall provide the temporary connections.
- 6. Determine the number and size of plugs to be used.

3.03 CLEANING PROCEDURE

Clean the pipeline using the following procedures and the Contractor's cleaning plan, as approved by the Engineer.

- A. Swab Cleaning Procedures
 - 1. Open the water supply upstream of the swab. Throttle the flow in the main at the discharge (plug exit) point so that the swab passes through the main at a speed of 2 to 4 fps. (At this velocity, swabs will effectively clean pipes for distances of up to 4,000 feet before disintegrating to a size smaller than the main.) Use pitot gauges at the exist hydrant or blow-off to estimate the flowrate in the main.
 - 2. Note the time of entry of the swab into the main and estimate its time of exit. If the swab does not reach the exit point in the estimated time plus ten minutes, then a blockage has probably occurred. Reverse the flow in the main and note the time required for the swab to reach the original entry point. From the return travel time, estimate the location of the blockage. The Engineer may require the use of a swab containing a transmitter to accurately locate the blockage.
 - 3. Swab repeatedly as needed. Stop swabbing when the water behind the swab emerging at the exit clears up within one minute. Account for all swabs inserted into the main.
 - 4. After the last swab has been recovered, flush the main to remove swab particles. This may require up to an hour of flushing.
- B. <u>Pig Cleaning Procedures</u>
 - 1. Remove all air valves along the line. Ensure that each isolating valves to the air release valve are completely closed. Operate system to prevent undesired build up of air while air release valves are out of service.

- 2. If the pig is inserted directly into the main, set it in motion by opening the upstream gate valve and a downstream fire hydrant or blow-off valve (usually the valve on the capped end at the exit point). If the pig is launched from a wye, fire hydrant, or other appurtenance, use an external pressurized water source to inject the pig into the main as described in Specification Section 3.01.
- 3. Once the pig is launched, control its speed by throttling the discharge at a downstream fire hydrant or blow-off. Operate pigs at the typical speed of 1 fps. This slow speed will help prevent pressure surges when the pig passes through undersized valves, enters smaller pipes, or turns through tees or crosses. Speeds of up to 2 fps. can be used on straight runs with no restrictions or sharp turns.
- 4. Make sufficient passes of the pig to obtain thorough cleaning. Two pigs may be used in tandem to save time and water. Sufficient cleaning is established when the water discharging after the pig becomes clear within one minute.

3.04 POST CLEANING PROCEDURE

After successful cleaning; test, flush, and disinfect the main in accordance with applicable sections of these Specifications.

PRESSURE AND LEAKAGE TESTS

PART 1: GENERAL

1.01 SCOPE OF WORK

Test all piping, valves, and appurtenances installed under these Contract Documents. Testing shall be performed concurrent with installation. Do not install more than 1,200 feet of pipe without being tested, unless approved by the Engineer.

1.02 SUBMITTALS

Prepare and submit schedules and procedures to the Engineer for testing of all parts of the water main installed in accordance with these Contract Documents. Submit the schedule at least seven days prior to any testing.

PART 2: PRODUCTS

2.01 EQUIPMENT

Contractor shall furnish the pump, gauges, metering devices, pipe connections, and all necessary apparatus for the pressure and leakage tests including gauges and metering devices. Excavate, backfill, and furnish all necessary assistance for conducting the tests.

American Water reserves the option to furnish the gauges and metering devices for the tests.

PART 3: EXECUTION

3.01 GENERAL

- A. Perform hydrostatic pressure and leak tests in accordance with AWWA C600, Section 5.2 - Hydrostatic Testing after the pipe or section of pipe has been laid, thrust blocking cured (min. 5 days), and the trench is completely or partially backfilled. Where practical, testing shall be performed fully isolated from the active distribution system.
- B. The Contractor may, at his option, completely backfill the trench or partially backfill the trench over the center portion of each pipe section to be tested. However, the Engineer may direct the Contractor to completely backfill the trench if local traffic or safety conditions require.
- C. For system operating pressures of 200 psi or less, perform the hydrostatic test at a pressure of no less than 100 psi above the normal operating pressure without exceeding the rating of the pipe and appurtenances. For system operating pressures in excess of 200 psi, perform the hydrostatic test at a pressure that is 1.5 times the normal operating pressure, but no more than the design rating of the pipe and appurtenances.

- D. Valves shall not be operated in either direction at a differential pressure exceeding the rated valve working pressure. A test pressure greater than the rated valve working pressure can result in trapped test pressure between the gates of a double-disc gate valve. For tests exceeding the rated valve working pressure, the test setup should include a provision, independent of the valve, to reduce the line pressure to the rated valve working pressure on completion of the test. The valve can then be opened enough to equalize the trapped pressure with the line pressure, or the valve can be fully opened if desired.
- E. The test pressure shall not exceed the rated working pressure or differential pressure of the valves when the pressure boundary of the test section includes closed, resilient-seated gate valves or butterfly valves.
- F. Attach a tapping sleeve and valve assembly to the main. Pressure test the assembly prior to making the tap. The required test pressure shall be determined in the same manner as for pipe. The test is acceptable if there is no pressure drop in 15 minutes at test pressure.

3.02 FILLING AND TESTING

- A. Slowly fill each segregated section of pipeline with water ensuring that all air is expelled. Extreme care must be taken to ensure that all air is expelled during the filling of pipe. The line shall stand full of water for at least twenty-four hours prior to testing to allow all air to escape. If necessary, tap the main at points of highest elevation to expel air as the pipe is filled. Remove the corporation stops and plug the taps after successfully filling the pipeline and expelling all air as approved by the Engineer.
- B. Apply the specified test pressure, measured at the point of lowest elevation, using a pump connected to the pipe in a manner satisfactory to the Engineer. If the elevation of the high point of the pipeline being tested is such that the pressure during testing will be below 85% of the required test pressure, the Engineer will require a separate test to be performed on this section of pipeline. In lieu of a separate test, the test pressure measured at the lowest elevation may be increased, within the pressure rating of the pipeline material, such that the resulting pressure at the highest point exceeds 85% of the required test pressure. The test will be conducted for at least two hours at the required test pressure ± 5 psi.
- C. Conduct a leakage test concurrently with the pressure test. Leakage is defined as the volume of the water that must be supplied into the newly laid pipeline to maintain pressure within 5 psi of the test pressure after it is filled and purged of air. Measure the volume of water using a calibrated container or meter.
- D. No pipeline installation will be accepted by the Engineer if the leakage is greater than that shown in the following table:

Avg. Test								Nomi	nal Pipo	: Diame	ter—in.			
psi psi	4	6	8	10	12	14	16	18	20	24	30	36	42	48
450	0.57	0.86	1.15	1.43	1.72	2.01	2.29	2.58	2.87	3.44	4.30	5.16	6.02	6.88
400	0.54	0.81	1.08	1.35	1.62	1.89	2.16	2.43	2.70	3.24	4.05	4.86	5.68	6.49
350	0.51	0.76	1.01	1.26	1.52	1.77	2.02	2.28	2.53	3.03	3.79	4.55	5.31	6.07
300	0.47	0.70	0.94	1.17	1.40	1.64	1.87	2.11	2.34	2.81	3.51	4.21	4.92	5.62
275	0.45	0.67	0.90	1.12	1.34	1.57	1.79	2.02	2.24	2.69	3.36	4.03	4.71	5.38
250	0.43	0.64	0.85	1.07	1.28	1.50	1.71	1.92	2.14	2.56	3.21	3.85	4.49	5.13
225	0.41	0.61	0.81	1.01	1.22	1.42	1.62	1.82	2.03	2.43	3.04	3.65	4.26	4.86
200	0.38	0.57	0.76	0.96	1.15	1.34	1.53	1.72	1.91	2.29	2.87	3.44	4.01	4.59
175	0.36	0.54	0.72	0.89	1.07	1.25	1.43	1.61	1.79	2.15	2.68	3.22	3.75	4.29
150	0.33	0.50	0.66	0.83	0.99	1.16	1.32	1.49	1.66	1.99	2.48	2.98	3.48	3.97
125	0.30	0.45	0.60	0.76	0.91	1.06	1.21	1.36	1.51	1.81	2.27	2.72	3.17	3.63
100	0.27	0.41	0.54	0.68	0.81	0.95	1.08	1.22	1.35	1.62	2.03	2.43	2.84	3.24

Allowable Leakage per 1000 ft. of Pipeline*---gph

*If the pipeline under test contains sections of various diameters, the allowable leakage will be the sum of the computed leakage for each size. The table has been generated from the formula: $L = \frac{S*D*P^{1/2}}{2}$ where L is the allowable leakage in gallons per hour, S is the length of 148,000 pipe in feet, D is the nominal pipe diameter in inches, and P is the test pressure in psig.

E. Should any test disclose damaged or defective materials or leakage greater than that permitted, the Contractor shall, at Contractor's expense, locate and repair and/or replace the damaged or defective materials. Materials used for repair must be approved by the Engineer and meet the specifications. Repeat the tests until the leakage is within the permitted allowance and is satisfactory to the Engineer.

DUCTILE IRON PIPE AND FITTINGS (Contractor Furnished)

PART 1: GENERAL

1.01 COORDINATION OF WORK

Connection to existing pipelines may require shutdown of American Water facilities. Closely coordinate construction work and connections with the American Water through the Engineer. The Engineer, in consultation with the American Water, may select the time for connection to existing pipelines, including Saturdays, Sundays, or holidays, which, in the opinion of the Engineer, will cause the least inconvenience to the American Water and/or its customers. Make such connections at such times as may be directed by the American Water, at the Contractor expense, with no claim for premium time or additional costs.

1.02 RELATED WORK

Section 15000 – Piping - General Provisions

1.03 SUBMITTALS

Submit shop drawings and manufacturer's literature for all Contractor supplied materials promptly to the Engineer for approval in accordance with Specification Section 01300.

1.04 REFERENCES

Refer to current AWWA Standards:

- A. C104 American National Standard for Cement-Mortar Lining for Ductile-Iron Pipe and Fittings for Water
- B. C105 American National Standard for Polyethylene Encasement for Ductile-Iron Pipe Systems
- C. C110 American National Standard for Ductile-Iron and Gray-Iron Fittings, 3-inch through 48-inch, for Water and Other Liquids
- D. C111 American National Standard for Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings
- E. C115 American National Standard for Flanged Ductile-Iron Pipe with Ductile-Iron or Gray-Iron Threaded Flanges
- F. C116 American National Standard for Protective Fusion-Bonded Epoxy Coatings for the Interior and Exterior Surfaces of Ductile-Iron and Gray-Iron Fittings for Water Supply Service
- G. C150 American National Standard for the Thickness Design of Ductile-Iron Pipe

- H. C151 American National Standard for Ductile-Iron Pipe, Centrifugally Cast, for Water
- I. C153 American National Standard for Ductile-Iron Compact Fittings, 3-inch through 24-inch and 54-inch through 64-inch, for Water Service
- J. C600 Standard for Installation of Ductile-Iron Water Mains and Their Appurtenances

PART 2: PRODUCTS

Research has documented that certain elastomers (such as those used in gasket material) may be subject to permeation by lower-molecular weight organic solvents or petroleum products. Products supplied under this Specification Section assume that petroleum products or organic solvents will not be encountered. If during the course of pipeline installation, the Contractor identifies, or suspects the presence of petroleum products or any unknown chemical substance, <u>notify the Engineer immediately</u>. Stop installing piping in the area of suspected contamination until direction is provided by the Engineer.

2.01 PIPE MATERIAL

A. General

- 1. Ductile iron pipe shall conform to the latest specifications as adopted by the American National Standards Institute, Inc., (ANSI) and the American Water Works Association (AWWA). Specifically, ductile iron pipe shall conform to AWWA C151.
- 2. The pipe or fitting exterior shall be coated with a bituminous coating in accordance with AWWA C151. The pipe or fitting interior shall be cement mortar lined and seal coated in compliance with the latest revision of AWWA C104.

B. <u>Quality</u>

Pipe and fittings shall meet the following minimum quality requirements by conforming to the following:

- 1. AWWA C104 / ANSI A21.4 Cement-Mortar Lining for Ductile-Iron Pipe and Fittings
- 2. AWWA C105 / ANSI A21.5 Polyethylene Encasement for Ductile-Iron Pipe Systems
- 3. AWWA C110 / ANSI A21.10 Ductile Iron and Gray Iron Fittings
- 4. AWWA C111 / ANSI A21.11 Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings
- 5. AWWA C115 / ANSI A21.15 Flanged Ductile-Iron Pipe with Ductile-Iron or Gray-Iron Threaded Flanges

- 6. AWWA C116 / ANSI A21.16 Protective Fusion-Bonded Coating for the Interior and Exterior Surfaces of Ductile-Iron and Gray-Iron Fittings
- 7. AWWA C150 / ANSI A21.50 Thickness Design of Ductile-Iron Pipe
- 8. AWWA C151 / ANSI A21.51 Ductile-Iron Pipe, Centrifugally Cast
- 9. AWWA C153 / ANSI A21.53 Ductile-Iron Compact Fittings

Ductile iron water pipe and fittings will be accepted on the basis of the Manufacturer's certification that the material conforms to this specification. The certification for iron fittings shall list a fitting description, quantity, bare fitting weight and source, (AWWA C110, C153 or Manufacturer, if fitting is not listed in either standard). The certification shall accompany the material delivered to the project site. The Owner reserves the right to sample and test this material subsequent to delivery at the project site. If foreign manufactured fittings are provided, then the Contractor is obligated to notify the Engineer with a submittal and provide the necessary documentation to satisfy the Engineer and the Owner that the materials provided meet the specified AWWA standards and, among other documentation that may be required, provide certificates of compliance on the component supplied.

C. Pipe Class

All pipe shall be thickness class 52.

D. <u>Testing</u>

Perform a hydrostatic test of all pipe and appurtenances as required by AWWA C151 and Specification Section 15030.

- E. Joints
 - 1. <u>Mechanical and Push-On</u> Mechanical and push-on joints including accessories shall conform to AWWA C111.
 - 2. Flanged

Flanged joints shall conform to AWWA C110 or ANSI B16.1 for fittings and AWWA C115 for pipe. Do not use flanged joints in underground installations except within structures.

Furnish all flanged joints with 1/8-inch thick, red rubber or styrene butadiene rubber gaskets. The bolts shall have American Standard heavy unfinished hexagonal head and nut dimensions all as specified in American Standard for Wrench Head Bolts and Nuts and Wrench Openings (ANSI B18.2). For bolts of 1-3/4-inches in diameter and larger, bolt studs with a nut on each end are recommended. The high-strength, low-alloy steel for bolts and nuts shall have the characteristics listed in Table 6 of AWWA C111. Exposed bolts and nuts in aggressive soils shall be Xylan or FluoroKote #1.

3. <u>Restrained Joint Pipe</u>

Restrained joints for pipes shall be of the boltless push-on type which provides joint restraint independent of the joint seal. Restrained push-on joints allowed for pipe only shall have accessories conforming to AWWA C111. Restrained system shall be suitable for the following minimum working pressures:

<u>Pressure (psi)</u>		
350		
300		
250		
200		

- F. Suppliers acceptable to American Water are:
 - United States Pipe & Foundry Co. 1101 East Pearl Street Burlington, NJ 08016
 - 2. Griffin Pipe Products Company 1100 West Front Street Florence, NJ 08518
 - McWane Cast Iron Pipe Co. P. O. Box 607 Birmingham, AL 35201
 - 4. American Cast Iron Pipe Company 2916 16h Street North Birmingham, AL 35207

2.02 FITTINGS

A. Ductile Iron Fittings

Standard fittings shall be ductile iron conforming to AWWA C110. Compact ductile iron fittings shall meet the requirements of AWWA C153.

1. <u>Working Pressures</u>

Fittings shall be suitable for the following working pressures unless otherwise noted in AWWA C110 or C153:

	Pressure	e (psi)
	Compact Fittings	Standard Fittings
<u>Size</u>	Ductile Iron	<u>Ductile Iron</u>
3" - 24"	350	250, 350 (with special gaskets)

30" - 48"	250	25	0
54" - 64"	150	N//	A

The use of standard ductile iron fittings having a 250-psi pressure rating with ductile iron pipe (having a rating of 350 psi) is not permitted except by the expressed written approval by the Engineer.

2. Coating and Lining

The fittings shall be coated on the outside with a petroleum asphaltic coating in accordance with AWWA C110 or fusion coated epoxy in accordance with AWWA C116 and lined inside with cement-mortar and seal coated in accordance with AWWA C104 or fusion coated epoxy in accordance with AWWA C116.

- B. Suppliers acceptable to American Water are:
 - (Sigma through) United States Pipe & Foundry Co. 1101 East Pearl Street Burlington, NJ 08016
 - (Tyler Union –domestic only) McWane Cast Iron Pipe Co. P. O. Box 607 Birmingham, AL 35201
 - American Cast Iron Pipe Company 2916 16h Street North Birmingham, AL 35207

C. Joints

1. Mechanical and Push-On

Mechanical and push-on joints including accessories shall conform to AWWA C111. Anti-Rotation I T-Bolts shall be used on mechanical joints shall be of domestic origin, high strength, low alloy steel bolts only, meeting the current provisions of American National Standard ANSI/AWWA C111/A21.1-90 for rubber gasket joints for cast iron or ductile iron pipe and fittings. Bolt manufacturer's certification of compliance must accompany each shipment. T-bolts shall be Xylan or FluoroKote #1, (corrosion resistant) to handle corrosive conditions on any buried bolts.

2. Flanged

Flanged joints shall meet the requirements of AWWA C115 or ANSI B16.1. Do not use flanged joints in underground installations except within structures. Furnish all flanged joints with a minimum 1/8-inch, thick red rubber or styrene butadiene rubber gasket. The bolts shall have American Standard heavy unfinished hexagonal head and nut dimensions all as specified in ANSI B18.2. Xylan or FluoroKote #1 Hex Bolts (corrosion resistant) to handle corrosive conditions shall be used on any buried flanged bolts. Flange gaskets shall be rubber in composition; paper gaskets are not permitted.

Bolts and nuts shall be threaded in accordance with ASME/ANSI B1.1, Unified Inch Screw Threads (UN and UNR Thread Form) class 2A external and class 2B internal. For bolts of 1-3/4-inches in diameter and larger, bolt studs with a nut on each end are recommended. Material for bolts and nuts shall conform to ASTM A307, 60,000 PSI Tensile Strength, Grade B, unless otherwise specified. Bolt manufacturer's certification of compliance must accompany each shipment.

3. Restrained

Restrained joints for valves and fittings shall be of the boltless push-on type which provides joint restraint independent of the joint seal. Field Lok gaskets are not permitted on valves or fittings. Restrained push-on joints allowed for pipe only shall have accessories conforming to AWWA C111. Restrained system shall be suitable for the following minimum working pressures:

Size	<u>Pressure (psi)</u>
Less than 20"	350
20"	300
24"	250
30" - 64"	250

Where adjacent fittings are to be placed (as in a mechanical joint hydrant tee and a mechanical joint hydrant valve), the use of a suitably sized Foster adaptor is permitted to facilitate restraint between the fittings.

PART 3: EXECUTION

3.01 INSTALLATION

Follow the provisions of Specification Section 15000 and 02210 in addition to the following requirements:

A. Push-On Joints

Clean the surfaces that the gasket will contact thoroughly, just prior to assembly using a bacteria free solution (bleach, potable water or NSF approved material). Insert the gasket into the groove in the bell. Apply a liberal coating of special lubricant to the gasket and the spigot end of the pipe before assembling the joint. Center the spigot end in the bell and push home the spigot end.

B. Mechanical Joints

Clean and lubricate all components with soapy water prior to assembly. Slip the follower gland and gasket over the pipe plain end making sure that the small side of the gasket and lip of the gland face the bell socket. Insert the plain end into socket. Push gasket into position with fingers. Seat gasket evenly. Slide gland into position, insert bolts, and tighten nuts by hand. Tighten bolts alternately (across from one another) to the recommended manufacturing rating or if not provided, to the following normal torques:

<u>Bolt Size</u>	Range of Torque (In Foot-Pounds)
5/8"	40 - 60
3/4"	60 - 90
1"	70 - 100
1-1/4"	90 - 120

After field installation, all bolts shall receive petrolatum tape or petroleum wax protection or other approved coating material. Protection shall be applied before applying polywrap per Specification 15131.

- C. <u>Restrained Joints</u>
 - 1. Ball and Socket

Assemble and install the ball and socket joint according to the manufacturer's recommendations. Thoroughly clean and lubricate the joint. Check the retainer ring fastener.

2. Push-On

Assemble and install the push-on joint according to the manufacturer's recommendations. Thoroughly clean and lubricate the joint. Check the retainer ring fastener.

Protect pipe from damage from the jacking device (backhoe bucket, pipe jack, etc.) when "pushing home" any pipe by using wood or other suitable (non-metallic) material.

3. Mechanical Joint

Assemble and install the mechanical joint according to the manufacturer's recommendations. Thoroughly clean and lubricate the joint. Use approved restrained joint device on fittings and valves where required and approved for use by Engineer.

D. Pipe Protection

Protect pipe from damage from the jacking device (backhoe bucket, pipe jack, etc.) when "pushing home" any pipe. Wood or other suitable material (non-metallic) shall be used to push home the pipe.

E. Gaskets

Gaskets shall be as provided or recommended by the manufacturer and satisfy AWWA C111 in all respects. As noted in the products section of this specification, some gasket materials are prone to permeation of certain hydrocarbons which may exist in the soil (see Part 2). Under these conditions and at the Engineer's discretion, Contractor is required to provide FKM (Viton, Flourel) gasket material in areas of concern.

STEEL PIPE AND FITTINGS (Contractor Furnished)

PART 1: GENERAL

1.01 COORDINATION OF WORK

Connection to existing pipelines may require shutdown of Owner facilities. Closely coordinate construction work and connections with the Owner through the Engineer. The Engineer, in consultation with the Owner, may select the time for connection to existing pipelines, including Saturdays, Sundays, or holidays, which, in the opinion of the Engineer, will cause the least inconvenience to the Owner and/or its customers, Make such connections at such times as may be directed by the Owner, at the Contract prices, with no claim for premium time or additional costs.

1.02 RELATED WORK

Section 15000 – Piping - General Provisions

1.03 SUBMITTALS

Submit shop drawings and manufacturer's literature for all Contractor supplied materials promptly to the Engineer for approval in accordance with Specification Section 01300.

PART 2: PRODUCTS

2.01 PIPE MATERIAL

All steel pipe shall be either fabricated pipe or mill pipe manufactured in accordance with AWWA C200 and the following:

Pipe up to and including 26-inch diameter shall be mill type pipe conforming to ASTM Specification A53, Type S, Grade B or fabricated pipe using ASTM A283, Grade C steel. Pipe larger than 26-inches shall be fabricated pipe using ASTM A283, Grade C steel with straight longitudinal welded seams.

The diameter specified is the nominal pipe size: the commercial designation or dimension by which the pipe is designated for simplicity.

Wall thicknesses for steel pipe and fittings shall be minimum 0.375 inches for pipe sizes up to eight (8) inches. For pipe diameters larger than eight (8) inches the minimum wall thickness shall be 0.5 inches. Heavier wall thickness, if required, will be specified in the Specification Special Conditions.

Furnish steel fittings that conform to AWWA C208.

Flanges shall be Class D, slip-on type in accordance with AWWA C207 (suitable for pressure ratings (maximum operating pressure plus surge) of 175 psi for sizes 4-12 inches

and 150 psi for sizes greater than 12 inches). Flanges shall be flat faced with O.D. and drilling in accordance with ANSI Standard B16.1.

Field welding of pipe shall be in accordance with AWWA C206.

Interior of all steel pipe and fittings shall be cement mortar lined in accordance with AWWA C205 unless water is identified as a soft, aggressive water that is incompatible with cement lining or interior epoxy lined in accordance with AWWA C210 (up to 24-inch diameter) or C213 (over 24-inch diameter). Field welded joints shall be lined in accordance with the AWWA C205.

Provide water stops for pipe and fittings where shown on the Drawings or as required to prevent leakage around the pipe fittings at penetrations between wet and dry areas.

The exterior of all buried steel pipe and fittings shall receive shop applied liquid adhesive and coal tar enamel protective coating in accordance with AWWA C203. Fusion-bonded epoxy coating in accordance with AWWA C213 is also acceptable.

PART 3: EXECUTION

3.01 INSTALLATION

Follow the provisions of Specification Section 15000 in addition to the following requirements:

- A. Locate pipe joints as shown on the Drawings except where field conditions dictate changes and such changes are approved by the Engineer. The Contractor shall be responsible for ensuring proper alignment and fit of all steel piping.
- B. Cut pipe for installing valves or fittings in a neat and workmanlike manner without damage to the pipe or lining. The end shall be smooth and at right angle to the axis of the pipe or properly beveled for the weld joint as required. Flame cutting of metal pipe is not permitted. All pipe cutting shall be at the Contractor's expense. Perform field welding of steel pipe in accordance with AWWA C206.

CONCRETE PIPE AND FITTINGS (Contractor Furnished)

PART 1: GENERAL

1.01 COORDINATION OF WORK

Connection to existing pipelines may require shutdown of Owner facilities. Closely coordinate construction work and connections with the Owner through the Engineer. The Engineer, in consultation with the Owner, may select the time for connection to existing pipelines, including Saturdays, Sundays, or holidays, which, in the opinion of the Engineer, will cause the least inconvenience to the Owner and/or its customers, Make such connections at such times as may be directed by the Owner, at the Contract prices, with no claim for premium time or additional costs.

1.02 RELATED WORK

Section 15000 – Piping - General Provisions

1.03 SUBMITTALS

Submit shop drawings and manufacturer's literature for all Contractor supplied materials promptly to the Engineer for approval in accordance with Specification Section 01300.

PART 2: PRODUCTS

Research has documented that certain pipe materials (such as polyvinyl chloride, polyethylene, and polybutylene) and certain elastomers (such as those used in gasket material) may be subject to permeation by lower-molecular weight organic solvents or petroleum products. Products supplied under this Specification Section assume that petroleum products or organic solvents will not be encountered. If during the course of pipeline installation the Contractor identifies, or suspects the presence of petroleum products or any unknown chemical substance, notify the Engineer immediately. Stop installing piping in the area of suspected contamination until direction is provided by the Engineer.

2.01 PIPE MATERIALS

A. <u>Prestressed Concrete Pressure Pipe, Steel Cylinder Type</u>

Prestressed concrete pressure pipe shall conform to the latest specifications as adopted by the American Water Works Association (AWWA). Specifically, prestressed concrete pressure pipe, steel cylinder type shall conform to AWWA C301, latest issue, except as modified herein.

The pipe shall be designed to meet the following requirements:

1. External Load

Depth of cover - 2-1/2 feet to 10 feet, whichever depth produces the greatest load in conjunction with live load.

Trench width - nominal pipe diameter plus 24 inches.

Soil density - 120 pounds per cubic foot.

Ku - 0.130

Laying condition - Type 2, per AWWA C600.

Trench (earth) load plus live load or truck per AWWA C150, (H-20, one truck, plus 1.5 impact).

If the depth of cover exceeds 10 feet, it will be so indicated on the Drawings and the pipe manufacturer shall design the pipe as required in these areas.

2. Internal Pressures

Working pressure - as per the Specification Special Conditions.

If not provided use 250 psig.

Surge pressure allowance - 100 psi.

3. Combined Loading

Concrete pipe shall be designed in accordance with AWWA C304-Design of Prestressed Concrete Cylinder Pipe, following the "limit states" method. Design parameters shall be subject to the approval of the Company.

B. Design Calculations

Submit design calculations to the Engineer for the most severe loading conditions for each size and class of pipe. If requested by the Engineer, submit calculations to support the design of any particular piece or pieces of pipe anywhere in the project. Any such design calculation will be provided as part of the Bid Price at no additional cost to the Owner.

Present the design calculations in a neat, readable form, with all fixtures, values and units included to facilitate ease of checking. Calculations shall include, but not be limited to, the following:

- 1. Pipe size
- 2. Cylinder thickness and area
- 3. Cylinder yield point and ultimate strength (astm designation)
- 4. Wire diameter or gauge
- 5. Wire area
- 6. Wire size
- 7. Wire spacing

- 8. Wire wrapping force
- 9. Ultimate strength of wire
- 10. Internal pressure at zero core compression (P_o)
- 11. Elastic limit pressure (P_L)
- 12. Burst strength (P_b)
- 13. 3-edge bearing load to produce incipient cracking
- 14. 9/10 of the three-edge bearing loading producing .001 inch crack in the core with no internal pressure (W_{o})
- 15. Ultimate 3-edge load
- 16. Resultant concrete compression
- 17. Gross wrapping stress in wire
- 18. Compressive strength of core concrete at time of wrapping
- 19. Core thickness
- 20. Outside coating thickness
- C. Testing

Check the design of each size and class of pipe by hydrostatic tests conducted on representative pipe in the manufacturer's shop to obtain the following actual test strengths:

- 1. The pressure to produce a surface crack in the coating of the barrel 0.001 inches wide by 12 inches long shall be at least $0.8 P_{\circ}$ for lined cylinder pipe or P_{\circ} for embedded cylinder pipe.
- 2. The ultimate pressure to produce burst shall be at least Pb.

Verify the design of each size and class of concrete cylinder pipe by conducting tests on representative specimens. The tests described in this paragraph are for proof of design only. It is not necessary that such tests be made on pipe manufactured specifically for this Contract. Certified reports covering tests made on other pipe of the same size, class, and design as specified herein and manufactured from materials of equivalent type and quality may be accepted as adequate proof of design.

D. <u>Joints</u>

Joints for concrete pipe and fittings shall be of the rubber gasket type meeting the requirements of AWWA C301. Prior to pipe shipment, protect the exposed steel portions of the joint rings on the completed pipe with a shop-applied rust-inhibiting primer or a metalized zinc coating. Joint lubricant shall be as recommended by the pipe manufacturer. Rubber gaskets shall meet the physical requirements of the appropriate ASTM Specifications.

2.02 FITTINGS

Fittings for prestressed concrete pressure pipe, steel cylinder type, shall be designed so as to be compatible with the pipe, and so as to provide at least equal resistance to internal and external loads of the pipe. Design criteria, joints, coatings and linings shall be as specified for the pipe. Fittings shall conform to the requirements of AWWA C301. Furnish adaptor units from concrete pipe to iron pipe or valves where necessary for proper connection.

Stock, without additional compensation, a number of bevel adaptors and short lengths of pipe at the job site to be used for diverting the main past obstructions or to make any changes in the line and grade of the main due to omissions on the laying schedule or tabulated layout.

Provide, without additional compensation, bevel pipe, outlet connections on straight pipe, closure-pieces, and other accessories as required to satisfactorily install the new main as shown on the plans.

2.03 COATING

Pipe and fittings shall have an exterior mortar coating as specified in AWWA C301.

PART 3: EXECUTION

3.01 INSTALLATION

Follow the general provisions provided in Specification Section 15000 in addition to the following:

Thoroughly clean the spigot end of the pipe before assembling a joint. Clean and lubricate the inside of the bell end of the pipe and the gasket with vegetable soap. Then, place the gasket around the spigot end so it is properly seated in the circumferential groove to maintain uniform tension in the gasket all around the pipe. Then, align the spigot end with the bell end of the pipe and insert it into the bell.

Check the joint to ensure that the gasket is in the proper position. Check the joints of pipe 24 inches or larger from the inside of the pipe. Before the spigot is thrust completely home, insert steel spacers in the seat of the bell to leave a half inch clearance. Insert a feeler gauge into the recess to check the position of the gasket. If the gasket cannot be felt all around the pipe, remove the spigot. The gasket may be reused if it is not damaged. However, relubricate both the gasket and the joint. After it has been determined that the gasket is in its proper position, remove the joint spacers and push or pull the pipe completely home. Check joints of pipe smaller than 24 inches from the outside of the pipe by inserting a feeler gauge into the flare of the bell to assure that no portion of the gasket is protruding.

After the joint is assembled, place a cloth band around the joint recess. Wire or strap it in position to provide a means of pouring grout in the recess. Pour a grout composed of one part cement and three parts sand into the joint recess beneath the band. Take measures to ensure that the entire recess around the pipe is completely filled.

Weld prestressed concrete pipe joints, when required by the Engineer, in accordance with American Water Works Service Company drawing No. 61-300-12SK entitled, "Specifications for Welding Prestressed Concrete Pipe Joints". This drawing is included at the end of this Specification Section.

POLYVINYL CHLORIDE (PVC) PIPE (Contractor Furnished)

PART 1: GENERAL

1.01 SECTION INCLUDES

PVC pressure pipe and fabricated fittings in nominal sizes 4 inches through 12 inches with cast iron pipe equivalent outside diameters. Under special conditions, 2-inch PVC may be specified in which case it shall meet NSF 61 and satisfy a 200-psi pressure rating.

1.02 SUBMITTALS

Submit manufacturer's product data, installation instructions and certification for all materials to be furnished in accordance with Specification Section 01300. Submit classification and gradation test results for embedment and pipe backfill material.

1.03 REFERENCES

- A. ASTM American Society for Testing and Materials
 - 1. A536 Standard Specification for Ductile Iron Castings.
 - 2. D2241 Standard Specification for Poly (Vinyl Chloride) (PVC) Pressure-Rated Pipe (SDR Series).
 - 3. D2855 Standard Practice for Making Solvent Cemented Joints with Poly (Vinyl Chloride) (PVC) Pipe and Fittings.
- B. AWWA American Water Works Association
 - 1. C605 Underground Installation of Polyvinyl Chloride (PVC) Pressure Pipe and Fittings for Water.
 - C900 Polyvinyl Chloride (PVC) Pressure Pipe, and Fabricated Fittings, 4 In. Through 48 In. (100 mm Through 1,200 mm), for Water Transmission Distribution.
 - 3. M23 PVC Pipe Design and Installation.

PART 2: PRODUCTS

Research has documented that certain pipe materials (such as polyvinyl chloride, polyethylene, and polybutylene) and certain elastomers (such as those used in gasket material) may be subject to permeation by lower-molecular weight organic solvents or petroleum products. Products supplied under this Specification Section assume that petroleum products or organic solvents will not be encountered. If during the course of pipeline installation, the Contractor identifies, or suspects the presence of petroleum products or any unknown chemical substance, <u>notify the Engineer immediately</u>. Stop installing piping in the area of suspected contamination until direction is provided by the Engineer.

2.01 PIPE MATERIALS

All PVC pipe shall be PVC 1120 pressure pipe made from class 12454 material as defined by ASTM D1784 with outside diameter dimensions of steel or cast-iron pipe. The PVC compounds shall be treated or certified suitable for potable water products by the National Sanitation Foundation (NSF) Testing Laboratory (NSF Standard No. 61).

PVC Pipe 2 inch (where permitted):

Pipe fittings less than 4-inches may be schedule 40 PVC (ASTM D2466) or schedule 80 PVC (ASTM D2467) systems having working pressures of 100 psi and 150 psi respectively.

PVC Pipe 4 inch through 12 inch:

AWWA C900, DR14 and where permitted DR18. DR25 pipe will not be allowed. PVC pipe has recently been upgraded by pressure class, however <u>American Water does not allow pipe in its system to be fully subject to the revised pressures in AWWA C900</u>. DR14 shall not be subjected to pressures exceeding 250 psi. When 2-inch PVC is provided it shall meet NSF 61 and be DR14 pipe. DR18 shall not be subjected to pressures exceeding 200 psi.

2.02 MANUFACTURERS

- A. PW Eagle, Inc. 1550 Valley River Drive Eugene, Oregon 97401 (541) 343-0200 www.pwpipe.com
- B. CertainTeed (restrained joint pipe only) Pipe & Plastics Group 750 East Swedesford Road Valley Forge, PA 19482 (800)274-8530 www.certainteed.com
- C. J-M Manufacturing Company, Inc.
 9 Peach Tree Hill Road
 Livingston, NJ 07039
 (973) 535-1633
 www.jmm.com

2.03 RECEIVING, HANDLING, AND STORAGE

- A. Inspect pipe and appurtenances for defects prior to installation in the trench. Set aside and clearly mark defective, damaged or unsound material and hold material for inspection by the Owner or Engineer.
- B. Load and unload all materials in accordance with the manufacturer's recommendations and in such a manner as to prevent damage. Do not drop pipe and accessories or handle then in a rough manner.

C. Provide safe storage for all materials. Cover stored pipe that will be exposed to sunlight for periods longer than 6 months. Cover with canvas or other opaque material with provision for adequate air circulation. PVC pipe shall not be stored close to heat sources, such as heaters, boilers, steam lines, or engine exhaust.

PART 3: EXECUTION

3.01 INSTALLATION

Follow the provisions of Specification Section 15000 and 02210 in addition to the following requirements:

- A. Remove all dirt and foreign matter from pipe before lowering it into the trench. Do not place debris, hand tools, clothing or other materials in the pipe. Keep pipe clean during and after laying.
- B. Lay pipe with the bell end pointing in the direction of work progress. Do not roll, drop or dump pipe or appurtenances into the trench.
- C. Assemble push-on joints in accordance with the pipe manufacturer's recommendations. Assemble mechanical joints in accordance with the fitting manufacturer's recommendations.
- D. Cut pipe with pipe saws, circular saws, handsaws, or similar equipment. Provide a smooth end at a right angle to the longitudinal axis of the pipe. Deburr, bevel, and re-mark insertion line on spigot ends. Match factory bevel length and angle for field bevels. When connecting to certain shallow depth bells, such as those on some cast iron fittings and valves, cut off the factory bevel and prepare a deburred, square cut end with a slight outer bevel.
- E. Clean the sealing surface of the spigot end, the pipe bell, the coupler or fitting, and the elastomeric gaskets immediately before assembly. Do not remove factory installed gaskets for cleaning. Keep the joint free of dirt, sand, grit, grease or any foreign material. Apply approved lubricant when assembling gasketed joints in accordance with the pipe manufacturer's requirements. The use of improper lubricants can damage gaskets. Excessive lubricant use can make disinfection more difficult and cause taste and odor problems when the line is placed in service.
- F. Good pipe alignment is essential for proper joint assembly. Align the spigot to the bell and insert the spigot into the bell until it contacts the gasket uniformly. Do not swing or "stab" the joint; that is, do not suspend the pipe and swing it into the bell. The spigot end of the pipe is marked by the manufacturer to indicate the proper depth of insertion. Avoid metal to plastic contact with the pushing the pipe home (use wood or other material to cushion moving the pipe. Neither deflection or bending of PVC pipe joints are permitted.
- G. Assemble pipe using the following types of joints:
 - 1. Gasketed bell joint Integral with the pipe or fitting
 - 2. Gasketed coupling A double gasketed coupling

- 3. Mechanical joint Any of the several joint designs that have gaskets and bolts manufactured in accordance with AWWA standards.
- H. Tracer Wire
 - 1. Place tracer wire in accordance with Specification 02558.
 - 2. The wire shall be contiguous except at test stations, valve boxes, and where splicing is required. All splices shall be encased with a 3M-Gel Pack Model No. 054007-09053.
- I. <u>Pressure testing of DR 14 PVC pipe should not exceed 250 psi. Pressure testing</u> of DR 18 PVC pipe should not exceed 200 psi if approved for use.
- J. PVC pipe fittings shall employ ductile iron pipe fittings per Specifications 15105 and 15106. See Standard Details for transitions between different pipe materials.
- K. <u>Gaskets</u>

Gaskets shall be as provided or recommended by the manufacturer and satisfy AWWA C111 in all respects. Where ductile iron pipe and PVC pipe are directly connected, the appropriate gasket material for this purpose shall be employed. As noted in the products section of this specification, some gasket materials are prone to permeation of certain hydrocarbons which may exist in the soil (see Part 2). Under these conditions and at the Engineer's discretion, Contractor is required to provide FKM (Viton, Flourel) gasket material in areas of concern.

3.02 SERVICE CONNECTIONS

- A. Install service connections in accordance with AWWA C605 and the manufacturer's recommendations using the following methods:
 - 1. Tapping is only permitted through the use of service clamps or saddles.
 - 2. Using injection molded couplings with threaded outlets.
 - 3. Tapping with large service connections through appropriately sized tapping sleeves and valves.
 - 4. Direct tapping of 1 inch and smaller service connections is not permitted. Use service saddles only for AWWA C900 pipe, for nominal pipe sizes 6 inch through 12 inch. Corporation stops shall be threaded and conform to AWWA C800.
 - 5. The distance between the PVC pipe joint and a service tap (2 inch and smaller) shall be a minimum of 3 feet. The distance between the PVC pipe joint and a service tap (4 inch and larger) shall be a minimum of 4 feet. Where necessary, excavate along the pipe to confirm the acceptable distance before starting the tap.

HIGH DENSITY POLYETHYLENE (HDPE) PIPE AND FITTINGS FOR WATER DISTRIBUTION AND TRANSMISSION (Contractor Furnished)

PART 1: GENERAL

1.01 SECTION INCLUDES

Furnishing and installing 4-inch through 63-inch high density polyethylene (HDPE) pipe and fittings for water distribution and transmission.

1.02 REFERENCES

- A. ASTM F1055 Standard Specification for Electrofusion Type Polyethylene Fittings for Outside Diameter Controlled Polyethylene Pipe and Tubing.
- B. ASTM D2683 Standard Specification for Socket-Type Polyethylene Fittings for Outside Diameter-Controlled Polyethylene Pipe and Tubing.
- C. ASTM D2774 Standard Practice for Underground Installation of Thermoplastic Pressure Piping.
- D. ASTM D3261 Standard Specification for Butt Heat Fusion Polyethylene (PE) Plastic Fittings for Polyethylene (PE) Plastic Pipe and Tubing.
- E. ASTM D3350 Standard Specification for Polyethylene Plastics Pipe and Fittings Materials.
- F. AWWA C906 Polyethylene (PE) Pressure Pipe and Fittings, 4 In. (100 mm) though 63 In. (1,575 mm), for Water Distribution and Transmission.

1.03 SUBMITTALS

Submit manufacturer's product data, installation instructions, and certification for all materials to be furnished in accordance with Specification Section 01300. Submit classification and gradation test results for material(s) to be used for pipe embedment and backfill.

PART 2: PRODUCTS

2.01 MATERIALS

A. Research has documented that certain pipe materials (such as polyethylene, polybutylene, polyvinyl chloride, and asbestos cement) and elastomers, such as used in jointing gaskets and packing glands, may be subject to permeation by lower molecular weight organic solvents or petroleum products. Products supplied under this Specification Section assume that petroleum products or organic solvents will not be encountered. If during the course of pipeline installation the Contractor identifies, or suspects the presence of petroleum products or any

unknown chemical substance, <u>notify the Engineer immediately</u>. Stop installing piping in the area of suspected contamination until direction is provided by the Engineer.

- B. Pipe and fittings shall be made from the same resin meeting the requirements of the Plastic Pipe Institute (PPI) material designation PE 3408 with an ATSM D3350 minimum cell classification of PE 345464C.
- C. The material shall have a minimum Hydrostatic Design Basis (HDB) of 1,600 psi at 73°F.
- D. All materials which come in contact with water, including lubricants, shall be evaluated, tested, and certified for conformance with ANSI/NSF Standard 61.

2.02 PIPE

- A. All pipe and fittings shall be manufactured in ductile iron pipe sizes (DIPS) only in accordance with AWWA C906.
- B. The pipe shall contain no recycled compound except for rework material generated in the manufacturer's own plant that has the same cell classification as the material to which it is being added. The pipe shall be homogeneous throughout and free of visible cracks, holes, voids, foreign inclusions, or other defects that may affect the wall integrity.
- C. Permanent identification of water piping service shall be provided by co-extruding longitudinal blue stripes into the pipe outside surface. The striping material shall be the same material as the pipe material except for color. Stripes printed or painted on the outside surface shall not be acceptable.
- D. The nominal pipe diameter is specified on the Contract Drawings. The DR (dimension ratio) and the pressure rating of the pipe shall be as noted on the plans.
- E. The minimum pressure rating will be 200 psi.
- F. HDPE may be deflected subject to approval by the Engineer. The following table shows maximum deflection based upon the allowable strain of the pipe wall. Potential flow restrictions, surge and other non- trench stability and pipe strain issues may reduce the values shown here per the Engineer's recommendations. The bend radius multiplier determines the minimum radius of the pipe curvature and is calculated by multiplying the outside diameter of the pipe by the multiplier from the appropriate DR used. Bending radius allowed by the manufacturer can vary. Verify the multiplier with the manufacturer. In no case shall the radius be less than 125% of the manufacturer's permitted multiplier.

PE Pipe Dimension Ratio (DR)	Allowable Deflection (percent)	Bend Radius Multiplier
32.5	8.1	50
26.0	6.5	45
21.0	5.2	40
19.0	4.7	37.5
17.0	4.2	32.5
15.5	3.9	30
13.5	3.4	27.5
11.0	2.7	25

2.03 FITTINGS

- A. Plain end butt fused fittings and electrofusion couplings shall be used when joining polyethylene materials. Mechanical (compression) fittings shall be used only when joining polyethylene materials to different piping materials and approved by the Engineer.
- B. The fittings shall contain no recycled compound except for rework material generated in the manufacturer's own plant that has the same cell classification as the material to which it is being added. The fittings shall be homogeneous throughout and free of visible cracks, holes, voids, foreign inclusions, or other defects that may affect the wall integrity.
- C. Butt fusion fittings shall comply with ASTM D3261.
- D. Electrofusion fittings shall comply with ASTM F1055.
- E. Mechanical (compression) fittings used with polyethylene pipe shall be specifically designed for, or tested and found to be acceptable for, use with polyethylene pipe.

2.04 ACCEPTABLE MANUFACTURERS

- A. CPChem Performance Pipe 5085 West Park Blvd., Suite 500 P.O. Box 269006 Plano, Texas 75093
- KWH Pipe Ltd.
 5225 Canyon Crest Drive Building 300, Suite 353 Riverside, California 92507

PART 3: EXECUTION

3.01 PACKAGING, HANDLING, AND STORAGE

- A. The manufacturer shall ensure that the interior of all pipe is clean and install plastic cleanliness plugs in all pipes to keep the pipe interiors clean. The manufacturer shall package the pipe in a manner designed to ensure that it arrives at the project neat, clean, intact, and without physical damage. The transportation carrier shall use appropriate methods and intermittent checks to assure that the pipe is properly supported, stacked, and restrained during transport such that the pipe is not nicked, gouged, or physically damaged.
- B. Inspect pipe and appurtenances for defects prior to installation in the trench. Set aside defective, damaged or unsound material and hold material for inspection by the Engineer.
- C. Pipe shall be stored on clean, level ground to prevent undue scratching or gouging. If the pipe must be stacked for storage, such stacking shall be done in accordance with the pipe manufacturer's recommendations. The pipe shall be handled in such a manner that it is not pulled over sharp objects or cut by chokers or lifting equipment.
- D. Sections of pipe having been discovered with cuts or gouges in excess of 10% of the pipe wall thickness shall be cut out and removed. The undamaged portions of the pipe shall be rejoined by butt fusing or the use of electrofusion fittings.

3.03 PIPE INSTALLATION

- A. Refer to Specifications 15000 and referenced drawings that are part of these Contract Documents. Trenching shall be performed in accordance with ASTM D2774 and embedment materials shall be in accordance with ASTM D2321.
- B. Remove all dirt and foreign matter from pipe before lowering into the trench. Do not place debris, hand tools, clothing or other materials in the pipe. Keep pipe clean during and after laying.
- C. Maximum pipe bending radius shall be in conformance with the manufacturer's recommendation for the specific diameter and dimension ratio (DR) of the pipe. Whenever possible, changes in direction shall be accomplished by bending the pipe in lieu of installing a fitting, except as approved by the Engineer.
- D. Place location wire immediately above the initial backfill material, directly over the pipe. The wire shall be contiguous except at test stations, valve boxes, and where splicing is required. All splices shall be encased with a 3M-Gel Pack model No. 054007-09053. Wire insulation shall be highly resistant to alkalis, acid and other destructive agents found in soil.
- E. Prevent flotation of sealed pipe during work stoppages.
- F. HDPE pipe will not be employed with directional drilling through rock and other abrasive conditions unless it is encased.

3.04 PIPE AND FITTING JOINING

- A. Butt fusion and electrofusion procedures shall be in accordance with the manufacturer's recommendations. Surfaces must be clean and dry before joining. The fusion equipment operator shall be fully trained in the use of the respective equipment. The wall thicknesses of the adjoining pipes shall have the same DR at the point of fusion.
- B. Butt fusion equipment shall be equipped with a Datalogger. Records of each weld (including, as a minimum, heater temperature, fusion pressure, and a graph of the fusion cycle) shall be appropriately identified and provided to the Engineer.
- C. Electrofusion reports of each weld shall be appropriately identified and provided to the Engineer. The reports shall include, as a minimum, the fusion date, time, ambient temperature, fitting type and size, user ID, and the manufacturer of the part.
- D. Mechanical (compression) joining of pipe and fittings is only permissible when joining polyethylene pipe to unlike materials. HDPE stiffeners shall be utilized with all mechanical (compression) fittings. Blocking must be provided at changes in direction for any mechanical fittings. Use of positive restrained joints fittings (nonfriction type) is permissible when approved by the Engineer.

3.05 SERVICE CONNECTIONS

- A. Sidewall fused polyethylene hot-tapping tees shall be used for 3/4 inch and 1 inch service lines off mains 3 inches to 12 inches in diameter. For larger sized mains, polyethylene service saddles may be used, sidewall fused, and then tapped with a tapping tool or machine.
- B. For large mains (>12 inch), mechanical clamps or tapping saddles may be used provided they are designed for HDPE pipe and acceptable to the manufacturer of the pipe.

3.06 TESTING AND DISINFECTION

A. Pressure testing shall be conducted in accordance with the Manufacturer's recommended procedure or as recommended by the Engineer. Pressure testing shall use water as the test media. Pneumatic (air) testing is prohibited. Air must be completely removed before pressure testing. Under no circumstances shall HDPE pipe be pressure tested when the temperature of the pipe is above 80°F.

PIPING SPECIALTIES (Contractor Furnished)

PART 1: GENERAL

1.01 SCOPE

This Specification Section covers the furnishing and installation of miscellaneous piping specialties as shown on the Drawings or as required to fulfill the intent of the project.

PART 2: PRODUCTS

2.01 POLYETHYLENE ENCASEMENT

- A. Polyethylene encasement shall conform to AWWA C105. The polyethylene film supplied shall be translucent and blue in color and distinctly marked (at minimum 2 foot intervals) with the following information:
 - 1. Manufacturer's name (or trademark)
 - 2. Year manufactured
 - 3. Minimum film thickness and material type (LLDPE or HDCLPE)
 - 4. Range of nominal pipe diameter size
 - 5. ANSI/AWWA C105/A21.5 (compliance)
 - 6. A warning "WARNING-CORROSION PROTECTION-REPAIR ANY DAMAGE"
 - 7. Labeled "WATER"
- B. Tape shall be polyethylene compatible adhesive and a minimum of 1.5-inch wide. Shall be Scotchwrap #50, Fulton #355, or Polyken #900.
- C. Store all polyethylene encasement out of the sunlight. Exposure of wrapped pipe should be kept to a minimum.
- D. Suppliers of polyethylene encasement shall be approved by the Owner.

2.02 VALVE BOXES

- A. All valves shall be provided with valve boxes of a design approved by the Engineer. Valve boxes shall be of the standard, adjustable, cast iron extension type, multiple piece, 5-1/4-inch shaft, screw type, and of such length as necessary to extend from the valve to finished grade. Cast iron valve boxes shall be hot coated inside and out with an asphaltic compound.
- B. Valve boxes shall be manufactured by one of the following approved manufacturers: Bingham & Taylor, Mueller, Handley Industries, A.Y. McDonald, Quality Water Products, or Clay and Bailey.
C. Valve box bases shall conform to the following:

Valve Size 4" and smaller	<u>Base</u> round, 8" in height, 10-7/8" diameter at bottom
6" and 8"	round, 11" in height, 14-3/8" diameter at bottom
10" and larger	oval, 11" in height, 15" x 11-1/8 diameter at bottom

2.03 RODS, BOLTS, LUGS AND BRACKETS

- A. All steel rods, bolts, lugs and brackets, shall be ASTM A36 or A307 carbon steel with xylan coating as a minimum requirement. The bolts shall have American Standard heavy unfinished hexagonal head and nut dimensions all as specified in ANSI B18.2. Xylan or FluoroKote #1 T-Bolts, corrosion resistant to handle corrosive conditions shall be used on any buried flanged bolts.
- B. After field installation, all steel surfaces shall receive a petrolatum wax tape coating in accordance with AWWA C217. Suppliers include, but are not limited to, Tapecoat® Envirotape® and Denso Densyl Tape. Surface preparation and tape installation shall be in accordance with ASTM C217 and the manufacturer's recommendations. Subject to approval by the Engineer, an alternative corrosion protection for exposed buried metal is an aerosol applied rubberized coating. The material shall be rapid dry and specifically designed for corrosion protection. 3M Rubberized Underseal Undercoating 08883 or any equivalent rubberized-bitumen based spray-on undercoating may be used. Follow manufacturer's recommendations for storage and application.

2.04 RETAINING GLANDS

- A. All retaining glands shall be ductile iron with ductile iron set screws. Pressure ratings for use with ductile iron pipe shall be a minimum of 250 psi. Retainer Glands shall be coated with electrostaticaly applied baked-on polyurethane coating or approved equal. Locking wedges, bolts, and set screws shall be coated with Xylan or FluoroKote #1.
- B. Retaining glands shall be manufactured by one of the following approved manufacturers:

EBBA Iron, Inc. PO Box 857 Eastland Texas 76448

2.05 TEST /TRACER BOXES

- A. All test/tracer boxes shall be 18-inch plastic box flared and squared at base and have a 4-inch I.D. with a 1 ½-inch cast iron flange. Lid shall be a one piece locking lid with "Test Station" marked on lid and shall contain 5 screw-type brass terminals on a non conductive terminal board.
- B. Test/tracer boxes shall be manufactured by one of the following approved manufacturers:

Handley Industries, Inc. 2101 Brooklyn Rd. Jackson, MI 49203 Model T-45

2.06 MARKING POSTS

- A. All marking posts shall be Rhino FiberCurve[™] with PolyTechCoating or equivalent fiber-composite marking posts. The color shall be standard blue for water and the length shall be a minimum of 66-inches. The decals be UV stable all weather type with a no dig symbol and white and contrasting white and blue vertical lettering: Butterfly and Gate Valves decals (Rhino GD-5226C) Blow-Offs decals (Rhino GD-5411C) Pipeline decals (Rhino GD-1333C).
- B. Marking Posts shall be manufactured by one of the following "approved manufacturers":

Rhino 280 University Drive Southwest Waseca, MN 56093 1-800-522-4343 Carsonite International 605 Bob Gifford Boulevard Early Branch, SC 29916 1-800-648-7916

PART 3: EXECUTION

3.01 INSTALLATION

Install "piping specialties" in accordance with the general provisions provided in Specification Section 15000 and the following:

A. Polyethylene Encasement

- 1. Encase piping in polyethylene as required to prevent contact with surrounding backfill and bedding material in all areas shown on the plans or designated by the Engineer. Polyethylene shall be 12 mils.
- 2. Install the polyethylene wrap material in accordance with the DIPRA Field Polyethylene Installation Guide and AWWA C105. Polyethylene shall fit snugly and not tightly stretched. All holes or tears shall be repaired with tape. Large holes or tears shall be repaired by taping another piece of polyethylene over the hole. Tape or plastic tie straps at joint overlaps and at every 3 foot interval.

- 3. Dig bell holes and slide polywrap over the adjacent pipe and provide a minimum of 1 foot of overlap. Tightly secure bottom of polywrap using two to three passes of polyethylene tape on the pipe to polywrap connection and the overlap polywrap to polywrap connection.
- 4. Where polyethylene wrapped pipe being installed connects to a pipe that is not wrapped (including existing pipe), extend the wrap a minimum of 3 feet onto the previously uncovered pipe. This includes service lines which may be wrapped in polyethylene or dielectric tape.
- 5. Exposure of wrapped pipe to sunlight should be kept to a minimum. Pipe can be stored with the polywrap on for a maximum of 30 days.
- 6. At no time shall the polywrapped pipe be subjected to a point load during handling, temporary storage, or installation. The polywrap must be moved away from the timbers or hoisting device while on the pipe to prevent point loads and resulting pin holes.
- 7. Direct service taps for polyethylene encased pipe shall follow the procedure described in AWWA Standard C600. Access to the main for tapping through polyethylene is accomplished by making two to three passes of polyethylene tape around the pipe and over the polywrap. The tap is to be made directly through the tape and polywrap.
- 8. Tape shall be polyethylene compatible adhesive and a minimum of 1.5" wide. Shall be Scotchwrap #50, Fulton #355, or Polyken #900.
- B. Valve Boxes

Valve boxes shall be supported so that no load can be transmitted from the valve box to the valve. See Detail Drawing 0201-0601-SD59. Install a self-centering alignment ring at the operating nut American Flow Control, or equal or otherwise make sure that the bottom of the box is centered over the operating and runs perpendicular to the horizontal.

1. Butterfly Valves and Gate Valves

Install the valves in strict accordance with the requirements of Specification Section 15000. Set valves at the required locations with joints centered, spigots home and valve stems plumb unless otherwise directed by the Engineer.

2. <u>Tapping Sleeves and Valves</u>

Install the valves in strict accordance with the requirements of Specification Section 15000. After installation of the tapping sleeve and valve assembly but prior to making the tap the assembly shall be pressure tested hydrostatically in accordance with Specification Section 15030. The test shall be made with the valve open using a tapped mechanical joint cap. No leakage is acceptable. The test pressure shall be maintained for 15 minutes minimum.

C. Air Release Valve Assembles

See Standard Detail Drawings for a typical air release valve assembly.

D. Air Blow-off

See Standard Detail Drawings for air blow-off details.

E. Corporations and Curb Stops

Service line piping shall be compatible with corporation and curbs stops provided with appropriate protection between dissimilar materials and a minimum of interconnecting fittings.

F. <u>Test/Tracer Wire Boxes</u>

Boxes shall placed at areas designated in the plans and shall be flush with existing grade unless otherwise noted.

G. Marker Posts

Install Marker Posts using equipment designed for its installation per manufacturer guidelines and place at locations noted in the drawings or as approved by Engineer.

GATE VALVES, PRESSURE REDUCING VALVES, CROSS CONNECTION AND BACKFLOW PREVENTION, and CHECK VALVES (Contractor Furnished)

PART 1: GENERAL

1.01 SCOPE

- A. Furnish, install, and test all gate valves, pressure reducing, and check valves shown on the Drawings or as directed by the Engineer.
- B. Conduct all Work in accordance with the Owner's Backflow and Cross Connection program. See related information in Appendix A of Section 01000.

1.02 SUBMITTALS

A. Submit shop drawings and manufacturer's literature to the Engineer for approval in accordance with Specification Section 01300.

1.03 RELATED WORK

A. Section 15000 – Piping - General Provisions.

PART 2: PRODUCTS

2.01 SMALL GATE VALVES

- A. All gate valves, 3 inches through 12 inches NPS, shall be iron body, resilient-seated, nut-operated, non-rising stem gate valves suitable for buried service. The valve interior and exterior shall be epoxy coated at the factory by the valve manufacturer in accordance with AWWA Standard C550 (average 6 mil minimum). The valves shall be designed for a minimum differential pressure of 250 psi and a minimum internal test pressure of 500 psi unless otherwise noted on the plans. Valves shall be designed to operate in the vertical position.
- B. Valves shall comply fully with AWWA Standard C509. Valve ends shall be push on joint or MJ (when restrained), or as shown on the plans or approved in writing in accordance with AWWA Standard C111. Stems shall be made of a low zinc alloy in accordance with AWWA C509 4.4.5. Stem seals shall be double O-ring stem seals. Square operating nuts conforming to AWWA Standard C509 shall be used. Valves shall open (left or right) in accordance with the Owner's standard. All valve materials shall meet the requirements of NSF 61.
- C. Test valves (Operation Test and Hydrostatic Tests) at the manufacturer's plant in accordance with AWWA Standard C509. Provide the Engineer with certified copies of all tests prior to shipment. The Engineer reserves the right to observe all tests.

- D. Acceptable manufacturers:
 - 1. Mueller Company: Model A-2360

2.02 LARGE GATE VALVES

- A. Gate valves larger than 12-inches NPS shall be iron body, double disc (metal to metal seat), parallel seats, bronze mounted, rubber O-ring packing seals, epoxy coated interior and exterior meeting the requirements of AWWA Standard C550, and conforming to AWWA Standard C500. Stems shall be made of a low zinc alloy in accordance with AWWA C500 4.4.9.6. All valves shall have openings through the body of the same circular area as that of the pipe to which they are attached. All valves furnished shall open (left or right) in accordance with the Owner's standard. All valve materials shall meet the requirements of NSF 61.
- B. Test valves (Operation Test and Hydrostatic Tests) at the manufacturer's plant in accordance with AWWA Standard C515. Provide the Engineer with certified copies of all tests prior to shipment. The Engineer reserves the right to observe all tests.
- C. Valves shall have mechanical joint ends unless otherwise designated on the plans or approved by the Engineer.
- D. The valves shall be designed for a minimum differential pressure of 150 psi and a minimum internal test pressure of 300 psi, unless otherwise noted on the plans. Make all valves tight under their working pressures after they have been placed and before the main is placed in operation. Any defective parts shall be replaced at the Contractor's expense.
- E. Acceptable manufacturer: Mueller Company.

2.03 DOUBLE CHECK VALVE BACKFLOW PREVENTION ASSEMBLY

- A. <u>Double Check Valve Backflow Prevention Assemblies shall be in accordance with AWWA C510-17.</u>
- B. Inlet and outlet of the assembly shall be threaded in accordance with ANSI/ASME B1.20.1 for taper pipe connections; or ANSI B16.24 for bronze flanges; or ANSI B16.1 for iron flanges; or ANSI/AWWA C660 for grooved and shoulder joints.
- C. Markings
 - 1. All markings shall be easily read and shall be stamped or cast on the body; or stamped, engraved or etched on a durable nameplate permanently attached to the body of the assembly and shall be located either:
 - a. on both sides of the body or
 - b. on a top surface of the body.
 - 2. Nameplates shall be either brass or stainless steel and affixed with stainless steel escutcheon pins. In attaching a nameplate or stamping data in the metal of the assembly, caution shall be exercised so as not to produce an area of stress concentration.
 - 3. Markings shall be permanent and not easily defaced.

- 4. All markings shall be in English units (i.e. psi, °F, inches).
- D. All assemblies which consist of independent units assembled for the purpose of preventing backflow shall comply with the material, the operational and the specifications as required for backflow assemblies. In order to ensure proper installations, all backflow prevention assemblies shall be delivered for installation completely assembled by the original manufacturer with all components as approved. Resilient seated shut-off valves and test cocks are considered integral parts of the assembly.
- E. There shall be no mechanical linkage between the two (2) check valves. Each check valve shall be free to operate independently through its entire movement. The movement of either the first or second check valve through its full limit of travel shall not affect the operations of the other check valve.

2.04 REDUCED-PRESSURE PRINCIPLE BACKFLOW PREVENTION ASSEMBLY

- A. <u>Reduced-Pressure Principle Backflow Prevention Assemblies shall be in</u> <u>accordance with AWWA C511-17.</u>
- B. Inlet and outlet of the assembly shall be threaded in accordance with ANSI/ASME B1.20.1 for taper pipe connections; or ANSI B16.24 for bronze flanges; or ANSI B16.1 for iron flanges; or ANSI/AWWA C660 for grooved and shoulder joints.
- C. Markings
 - 1. All markings shall be easily read and shall be stamped or cast on the body; or stamped, engraved or etched on a durable nameplate permanently attached to the body of the assembly and shall be located either:
 - a. on both sides of the body or
 - b. on a top surface of the body.
 - 2. Nameplates shall be either brass or stainless steel and affixed with stainless steel escutcheon pins. In attaching a nameplate or stamping data in the metal of the assembly, caution shall be exercised so as not to produce an area of stress concentration.
 - 3. Markings shall be permanent and not easily defaced.
 - 4. All markings shall be in English units (i.e. psi, oF, inches).
- D. A backflow prevention assembly shall be designed so that each principle component (i.e. check valve if a separate body and the differential pressure relief valve) of the assembly shall be permitted to be removed and reinstalled individually from the line.
- E. A backflow prevention assembly shall be provided with one or more openings through which the internal parts may be removed, repaired or inspected without having to remove the body of the assembly from the line.
- F. In the RPZ Assembly each of the check valves shall likewise be free of any mechanical linkage and shall be free to operate independently through its entire movement. The relief valve shall be mechanically independent of both check valves yet hydraulically dependent upon the pressure differential across the fist

check valve. The movement of both the first or second check valves and the relief valve through their full limits of travel shall not affect the operations of the other check valve.

G. The differential pressure relief valve of a RPPA shall be located so that its valve seat(s) and port(s) to the atmosphere are below the lowest point of the first check valve so as to preclude backsiphonage.

2.05 CHECK VALVES

Check valves shall be tilted disc check valves conforming to the following requirements:

- A. Construction
 - 1. Check valves shall be manufactured from gray iron meeting or exceeding ASTM A126 Grade B.
 - 2. Check valves are to comply with ANSI/AWWA C508, latest revision for use in water works service, and at a rated working pressure of 200 psig. Test pressure shall be at 400 psig.
 - 3. Check valves shall be Certified to NSF/ANSI 372.
 - 4. The disc on check valves 4 in. and larger shall be made of gray iron with a bronze disc ring securely fastened into grooves machined in the disc.
 - 5. Valve body shall a threaded-in bronze seat ring.
 - 6. The connection between the disc and clapper arm shall be designed with sufficient clearance to allow the disc to adjust to the seat.
 - 7. The clapper arm shall be made of ductile iron conforming ASTM A536 and shall exhibit a be bronze-bushed design to accommodate the disc assembly.
 - 8. Clapper arm shaft shall be made of a high tensile strength, corrosion resistant stainless steel.
 - 9. Clapper arm assembly shall mate with the valve body by using mounted bronze retaining plugs.
 - 10. In applications requiring a lever and weight, or lever and spring design, where the clapper arm shaft extends outside the valve body, a double Oring seal fully contained within the shaft bearing shall be provided. The lever and shaft design shall employ the use of a grease fitting for lubrication between the O-rings.

B. Disc Feature

- 1. Disc shall not contact the body when the valve is in the full open position and shall swing clear of the waterway. Disc shall remain in the closed position when installed in a horizontal pipeline under no-flow condition.
- 2. Disc shall exhibit a clear-waterway as defined by ANSI/AWWA C508 and a Type 3 clear-waterway as defined by MSS SP-71.

C. Coatings

- 1. Valves to be coated internally with a NSF 61 Certified coating.
- 2. Exterior shall be coated with a primer suitable for field application of a comparable top coat.

D. <u>Useage</u>

- 1. Valve to be designed for use in water and wastewater applications. Check valves equipped with lever and weight, or lever and spring, can be used in a horizontal pipeline, or in a vertical pipeline only when flow is in an upward direction. The lever orientation shall be field adjustable and must be positioned correctly for the installation.
- E. <u>Acceptable manufacturer:</u> AMERICAN Flow Control's Series 52-SC Swing Check Valve with Strainer

2.06 PRESSURE REDUCING VALVES AND VAULTS

- A. <u>Control Valves:</u> Control valves include, but are not limited to, altitude valves, pressure reducing valves, and backpressure sustaining valves. Altitude valves shall be single-acting altitude valves with an internal stop-check feature. Valve bodies shall be cast iron or cast ductile iron.
- B. <u>Manufacturer:</u> Subject to compliance with requirements, provide products from one of the manufacturers listed below:
- C. <u>Water Pressure Reducing Valves shall be supplied by Cla-Val Co. or equal and comply with the following requirements:</u>
 - 1. Pressure reducing valves are to comply with ANSI/AWWA C530
 - 2. The pressure reducing valves body and cover shall be made of ductile iron conforming ASTM A536.
 - 1. 400 psi max pressure rating
 - 2. Lead free NSF/ANSI 61
 - 3. Fusion bonded epoxy coating (Interior and Exterior); ANSI / NSF 61 Approved / AWWA coating specifications C116-03
 - 4. Integral mesh strainer (1000 micron)
 - 5. Pressure gauge port on both sides of valve body
 - 6. Nylon Reinforced Buna-N® Rubber diaphragm
- D. <u>Basis of Design</u>: The control valve shall be CLA-VAL Company Model No. #90G-01, Pressure Reducing Control Valve with X141 gauges – Outlet Pressure, as manufactured by Cla-Val Co.
- E. <u>Pressure Reducing Valve (PRV) Vault:</u> Use Concrete Pipe and Precast's Clear Flow PRV Valve detail. Detail shown on plans and provided with these specifications. Clear Flow Vault shipped assembled weighing approximately 48,000 lbs.

PART 3: EXECUTION

3.01 OPERATION

All valves (less than 12-inches diameter) shall be operated by the Owner unless under the direct supervision of an employee of the Owner. All valves 12-inches diameter and greater shall be operated by the Owner.

3.02 INSTALLATION

- A. Install the valves in strict accordance with the requirements contained in Specification Section 15000 and detail drawings. All large gate valves shall be restrained.
- B. Install all valves in accordance with local State and City/County water works regulations.
- C. The system shall be designed to maintain a minimum pressure of 20 psi in the distribution system at all service connections at the design flow. Design flow shall be defined as the greater of maximum hour or maximum day plus applicable fire flows. Where the pressure at the service tap exceeds 80 psi a reducing valve shall be required in accordance with the provisions of the Uniform Statewide Building Code. Reducing valves shall be installed on the downstream side of the water meter and shall be the responsibility of the property owner. Provisions shall be made on the downstream side of the pressure reducing valve for pressure build up due to water expansion and contain a pressure relief valve to atmosphere.
- D. An approved backflow prevention device shall be installed at the end of each service connection to the consumer's water system serving, but not limited to the following:
 - 1. Hospitals, Mortuaries, Clinics, Veterinary establishments, Medical buildings and laboratories
 - 2. Piers docks and waterfront facilities
 - 3. Sewage treatment plants sewage pumping stations or storm water pumping stations
 - 4. Food and beverage processing plants
 - 5. Chemical plants, dyeing plants and pharmaceutical plants
 - 6. Metal plating industries
 - 7. Petroleum or natural gas processing or storage plants
 - 8. Radioactive materials processing plants or nuclear reactors
 - 9. Car washes and laundries
 - 10. Lawn sprinkler systems and irrigation systems
 - 11. Fire Service systems
 - 12. Slaughter houses and processing plants
 - 13. Farms where the water is used for anything other than typical house hold use
 - 14. Green houses and nurseries
 - 15. Health Clubs with swimming pools, therapeutic baths, hot tubs or saunas
 - 16. Paper and paper products plants and printing plants
 - 17. Pesticide or exterminating companies
 - 18. Any vehicles with storage or mixing tanks
 - 19. Schools with laboratory facilities
 - 20. High rise building (4 units or more)
 - 21. Multi use commercial, office or warehouse facilities
 - 22. Others specified by the purveyor or the division when reasonable cause can be shown for a potential backflow or cross connection hazard

- E. Facilities having booster pumps shall be equipped with a low suction pressure cutoff device to shut off the pump when the pressure in the waterworks drops to a minimum of 10 psi.
- F. Where water is supplied to fixtures or systems deemed high hazard, the device providing service line protection is typically required to be one using the reduced pressure zone principle. High hazard fixtures and systems are associated with the following facilities, among others:
 - 1. Multi-use commercial, office, or warehouse facilities
 - 2. High rise buildings (four or more stories)
 - 3. Lawn sprinkler systems and irrigation systems
 - 4. Fire suppression systems with chemical additives
 - 5. Hospitals, mortuaries, clinics, veterinary establishments, nursing homes, and medical buildings
 - 6. Laboratories, and schools or colleges with laboratory facilities
 - 7. Sewage treatment plants, sewage pumping stations or water pumping stations
 - 8. Food and beverage processing plants
 - 9. Health clubs with swimming pools, therapeutic baths, hot tubs or saunas
 - 10. Metal plating industries
 - 11. Petroleum or natural gas processing or storage plants
 - 12. Car washes and laundries
 - 13. Pesticide or exterminating companies, and associated vehicles with storage or mixing tanks
 - 14. Farms where water is used for purposes other than typical household use
 - 15. Commercial greenhouses and nurseries

3.03 PROTECTION

After field installation of the valve all external bolts except the operating nut shall receive a layer of tape coating or approved rubberized-bitumen based spray-on undercoating applied before backfill. If polyethylene is applied to the pipe, the entire valve shall be encased in polyethylene encasement prior to backfill. The polyethylene encasement shall be installed up to the operating nut leaving the operating nut exposed and free to be operated. Valve box shall be installed per Piping Specialties Section 15130 or 15131.

BUTTERFLY VALVES (Contractor Furnished)

PART 1: GENERAL

1.01 SCOPE

Furnish and install all butterfly valves shown on the Drawings and/or the Specification Special Conditions.

1.02 RELATED WORK

Section 15000 – Piping - General Provisions.

1.03 SUBMITTALS

Submit shop drawings and manufacturer's literature to the Engineer for approval in accordance with Specification Section 01300.

PART 2: PRODUCTS

2.01 VALVES

- A. Furnish and install rubber-seated butterfly valves as shown on the Contract Drawings. Butterfly valves shall conform to Class 150B of the AWWA Standard C504 and this specification unless working pressure is greater than 150 psi. In which case, the butterfly valve shall conform to Class 250B of the AWWA Standard C504. All valves furnished shall open (left or right) in accordance with the Owner's standard.
- B. Valve bodies shall be ductile iron with mechanical joint ends. Mechanical joint ends shall conform to AWWA Standard C111. All valve materials shall meet the requirements of NSF 61.
- C. Valve shafts shall consist of one-piece units extending through the discs of 18-8 stainless steel Type 303 or 304. Shaft diameter shall be in accordance with Table 3 of AWWA Standard C504.
 - 1. Valve discs shall be Ni-Resist, Type 1, or cast iron with stainless steel edges.
 - 2. Valve seats shall be hycar or natural rubber mounted in the valve body.
 - 3. Valve bearings shall be nylon or Teflon.
- D. The valve interior and exterior shall be epoxy coated at the factory by the valve manufacturer in accordance with AWWA Standard C550 (6-8 mil average, 4 mil minimum).

- E. All elastomers used in the butterfly valves must be suitable for service in the following water conditions:
 - 1. Chlorine concentration up to 12 mg/L
 - 2. Chloramine concentrations up to 6 mg/L
 - 3. Ozone concentrations up to 2.0 mg/L (AWWA Standard says 0.5 ppm) pH range of 4-11
- F. Manual buried operators, if provided, shall be either worm gear or traveling nut type and shall be furnished with 2-inch AWWA nuts and extension shafts. Input required at nuts to produce specified output torque shall be less than 150 ft.-lbs. Operators shall be designed to withstand an input at the nut of 300 ft.-lbs. without damage to any operator components.
- G. Acceptable manufacturers:
 - 1. Mueller Company (Henry Pratt Company Division only)
 - 2. DeZurik Water Controls

PART 3: EXECUTION

3.01 SETTING VALVES

Install the valves in strict accordance with the requirements of Specification Section 15000. All butterfly valves shall be restrained.

3.02 PROTECTION

After field installation of the valve all external bolts except the operating nut shall receive a layer of tape coating or approved rubberized-bitumen based spray-on undercoating applied before backfill. If polyethylene is applied to the pipe, the entire valve shall be encased in polyethylene encasement prior to backfill. The polyethylene encasement shall be installed up to the operating nut leaving the operating nut exposed and free to be operated.

TAPPING SLEEVES, SADDLES, AND VALVES (Contractor Furnished)

PART 1: GENERAL

1.01 SCOPE

Furnish, install and test all tapping sleeves, tapping valves, and tapping saddles as shown on the Drawings.

1.02 RELATED WORK

Section 15000 – Piping - General Provisions

1.03 SUBMITTALS

Submit shop drawings and manufacturer's literature to the Engineer for approval in accordance with Specification Section 01300.

PART 2: PRODUCTS

2.01 GENERAL

All tapping sleeves, saddles and valves shall be designed for a working pressure of at least 250 psig for 12-inch and smaller. The valves shall be designed for a minimum differential pressure of 250 psi and a minimum internal test pressure of 500 psi unless otherwise noted on the plans.

2.02 DUCTILE IRON TAPPING SLEEVES

Verify the type of existing pipe and the outside diameter of the pipe on which the tapping sleeve is to be installed.

Tapping sleeves shall be ductile iron dual compression type unless otherwise specified on the Drawings. The Drawings may require the use of corrosion resistant tapping sleeves in addition to polywrap in areas with corrosive soils. The sleeves shall be made in two halves which can be assembled and bolted around the main. Sleeves shall meet the requirements of NSF 61. Outlet flanges shall conform to the flange requirements of AWWA C110. All valves furnished shall open (left or right) in accordance with the Owner's standard.

Acceptable manufacturer: Mueller Company Model T-2360

2.03 TAPPING VALVES

The horizontal tapping valve shall conform to the applicable requirements of AWWA Standard C509. All tapping valves, 3 inches through 12 inches NPS, shall be ductile iron body, resilient-seated, nut-operated, non-rising stem gate valves suitable for buried service. The valve interior and exterior shall be epoxy coated at the factory by the valve manufacturer in accordance with AWWA Standard C550 (6-8 mil average, 4 mil minimum).

The tapping valves shall have flanged inlets with mechanical joint outlets, enclosed bevel gears, bypass valve, rollers, tracks and scrapers. All valves furnished shall open (left or right) in accordance with the Owner's standard.

Acceptable manufacturer:

- Mueller Company Model H-615
- American Flow Control Series 2800-C

2.04 STAINLESS STEEL TAPPING SLEEVES

The stainless steel band flange shall be manufactured in compliance with AWWA C207, Class D ANSI B.16.1 drilling, recessed for tapping valve MSS-SP60. Mechanical Joint tapping sleeve outlet shall meet or exceed all material specifications as listed below and be suitable for use with standard mechanical joint by mechanical joint resilient wedge gate valves per ANSI/AWWA C509 and be NSF 61 approved.

A. <u>Tapping sleeves from 4-inch through 12-inch</u>

Tapping sleeves to be attached to 4-inch through 12-inch nominal pipe diameter shall meet the following minimum requirements:

- The entire fitting shall be stainless steel type 304 (18-8). The body, lug, and gasket armor plate shall be in compliance with ASTM A240. The Flange shall be cast stainless steel in compliance with ASTM A743. The MJ outlet shall be one-piece casting made of stainless steel. The test plug shall be ³/₄-inch NPT in compliance with ANSI B2.1 and shall be lubricated or coated to prevent galling. All metal surfaces shall be passivated after fabrication in compliance with ASTM A-380.
- 2. The gasket shall provide a 360-sealing surface of such size and shape to provide an adequate compressive force against the pipe after assembly, to affect a positive seal under the combinations of joint and gasket tolerances. The materials used shall be vulcanized natural or vulcanized synthetic rubber with antioxidant and antiozonant ingredients to resist set after installation. No reclaimed rubber shall be used. A heavy-gauge-type 304-stainless armor plate shall be vulcanized into the gasket to span the lug area.
- 3. The lugs shall be heliarc welded (GMAW) to the shell. The lug shall have a pass-through-bolt design to avoid alignment problems and allow tightening from either side of the main. Bolts shall NOT BE integrally welded to the sleeve. Finger Lug designs are not approved; it is the intent of these specifications to allow a tapping sleeve that has a lug design similar to the approved models.
- 4. Bolts and nuts shall be type 304 (18-8) stainless steel and Teflon coated or as specified in the bolt section below at the discretion of the Engineer. Bent or damaged units will be rejected.
- 5. Quality control procedures shall be employed to ensure that the shell, lug, (4-inch and Larger Nominal Pipe Diameter) armor plate, gasket and related hardware are manufactured to be free of any defects. Each unit, after proper installation, shall have a working-pressure rating up to 250 psi.

- 6. The sleeve construction shall provide a positive means of preventing gasket cold flow and/or extrusion.
- 7. Each sleeve shall be stenciled, coded or marked in a satisfactory manner to identify the size range. The markings shall be permanent type, water resistant, that will not smear or become illegible.

B. <u>Tapping sleeves from 16-inch and larger</u>

Tapping sleeves attached to 16-inch and larger nominal pipe diameter shall meet the following minimum requirements:

- 1. The body shall be in compliance with ASTM A285, Grade C or ASTM A36. The test plug shall be $\frac{3}{4}$ " NPT conforming to ANSI B2.1.
- 2. The gasket shall provide a watertight sealing surface of such size and shape to provide an adequate compressive force against the pipe. After assembly, the gasket will insure a positive seal under all combinations of joint and gasket tolerances. Gaskets shall be formed from vulcanized natural or vulcanized synthetic rubber with antioxidant ingredients to resist set after installation. No reclaimed rubber shall be used.
- 3. Bolts and nuts shall be high strength, corrosion resistant, low alloy, pre AWWA C111, ANSI A21.11 and as specified in the subsection on bolts in this specification.
- 4. Quality control procedures shall be employed to ensure that the shell, gaskets, and related hardware area are manufactured to be free of visible defects. Each unit, after proper installation, shall have a working-pressure rating up to 200 psi.
- 5. Unless otherwise noted, unit shall be protected by electrostatically applied baked epoxy or polyurethane.
- 6. Units for concrete, steel cylinder pipe shall be furnished with load bearing setscrews on the gland flange to transfer loads on the outlet away from the steel cylinder and onto the sleeve. Epoxy –coated tapping sleeves do not require grout seal cavity (AWWA M-9 Manual).
- 7. Each sleeve shall be stenciled, coded or marked in a satisfactory manner to identify the size range. The marking shall be permanent type, water resistant, that will not smear or become illegible.

2.05 FABRICATED STEEL TAPPING SLEEVES

The fabricated steel tapping sleeve shall be manufactured in compliance with AWWA C207. Sleeves shall be fabricated of minimum three-eights (3/8) inch carbon steel meeting ASTM A285 Grade C. Outlet flange shall meet AWWA C-207, Class "D" ANSI 150 lb. drilling and be properly recessed for the tapping valve. Bolts and nuts shall be high strength low alloy steel to AWWA C111 (ANSI A21.11). Gasket shall be vulcanized natural or synthetic rubber. Sleeve shall have manufacturer applied fusion bonded epoxy coating, minimum 12 mil thickness, Class D ANSI B.16.1 drilling, recessed for tapping valve MSS-SP60. Mechanical Joint tapping sleeve outlet shall meet or exceed all material specifications as listed below and be suitable for use with standard mechanical joint by mechanical joint resilient wedge gate valves per ANSI/AWWA C509-94 and be NSF 61 approved.

2.06 TAPPING SADDLES

Unless otherwise specified by the Drawings, tapping saddles conform to the requirements of AWWA Standard C800 for the High-Pressure class tapping saddles. Tapping saddles shall consist of ductile iron outlet castings, attached to the pipeline with high strength stainless steel straps. Castings shall be sealed to pipeline with O-ring seals. Saddles shall have ANSI A21.10 flanged outlets counterbored for use with tapping valves and tapping equipment.

2.07 BOLTS

All bolts shall have American Standard heavy unfinished hexagonal head and nut dimensions all as specified in ANSI B18.2. Bolts shall be Xylan or FluoroKote #1 suitable for direct bury in corrosive soils.

PART 3: EXECUTION

3.01 INSTALLATION

Install the tapping sleeves, saddles, and valves in strict accordance with the requirements of Specification Section 15000. Install the tapping sleeves, tapping saddles, and tapping valves in accordance with the manufacturer's instructions. The tapping procedure is to be in accordance with the tapping machine manufacturer's instructions.

3.02 PROTECTION

After field installation of the valve all external bolts except the operating nut shall receive a layer of tape coating or approved rubberized-bitumen based spray-on undercoating applied before backfill. If polyethylene is applied to the pipe, the entire sleeve and valve assembly shall be encased in polyethylene encasement prior to backfill. The polyethylene encasement shall be installed up to the operating nut leaving the operating nut of the tapping valve exposed and free to be operated.

3.03 PRELIMINARY TESTING

Perform a hydrostatic test of the tapping sleeve and valve assembly in accordance with Specification Section 15030 after installation of the tapping sleeve and valve, but prior to making the tap. The test shall be made with the valve open using a tapped mechanical joint cap. No leakage is acceptable. The test pressure shall be maintained for a minimum of 15 minutes.

Perform hydrostatic test of tapping saddles in accordance with AWWA Standard C800.

FIRE HYDRANTS (Contractor Furnished)

PART 1: GENERAL

1.01 SCOPE

Furnish all labor, material, tools, and equipment required to install fire hydrants at the location shown on the plans, or where designated by the Engineer.

PART 2: PRODUCTS

2.01 MATERIAL

- A. All fire hydrants shall be ductile iron and conform to the requirements of AWWA C502, traffic-model break-away type fire hydrants.
- B. Contact the local water district and obtain written fire hydrant mechanical details for the water district prior to ordering any fire hydrants for the Work. All fire hydrants shall open left or right as required and be clearly marked on the top of the hydrant with a 1 ½-inch pentagon top nut and have not less than two (2) O-ring stem seals. The number and sizes of hose nozzle outlets is dependent on the local regulation. The hydrant shall be break-away traffic flange. The hydrant interior and exterior shall be epoxy coated at the factory by the hydrant manufacturer in accordance with AWWA Standard C550 (6-8 mil average, 4 mil minimum). The Contractor shall contact the local water district and obtain written fire hydrant mechanical details for the water district prior to ordering any fire hydrants in accordance with the drawings.
- C. All hydrant materials shall meet the requirements of NSF 61.
- D. Acceptable manufacturers and models, subject to the specifications set forth, include:
 - 1) Mueller Super Centurion 250
 - a. Dale City: Mueller Model #A423-500-087
 - b. Alexandria City: Mueller Model #A423-500-100

PART 3: EXECUTION

3.01 INSPECTION PRIOR TO INSTALLATION

- A. Contractor shall inspect all fire hydrants upon receipt. Cycle each hydrant to full open and full closed positions to ensure that no internal damage or breakage has occurred during shipment and handling. Check all external bolts for proper tightness.
- B. After inspection, close the hydrant valves and replace the outlet nozzle caps to prevent the entry of foreign matter. Protect stored hydrants from the weather/elements with the inlets facing downward.

3.02 INSTALLATION

- A. Locate hydrants on the plans or as directed by the Engineer and in compliance with local regulations. The location shall provide complete accessibility and minimize the possibility of damage from vehicles or injury to pedestrians. When placed behind the curb, the hydrant barrel shall be set so that no portion of the pumper or hose nozzle cap will be less than 18 to 24 inches, depending on local requirements, from the gutter face of the curb. All hydrants shall stand plumb with the pumper nozzle facing the curb. Set hydrants with nozzles at least eighteen inches above the finished grade as shown on the plans. Set the break flange at least 2 but no more than 6 inches above finished grade, or as directed by the Engineer. Connect each hydrant to the main with a 6-inch branch connection controlled by an independent 6-inch gate valve, unless otherwise shown on the plans. All hydrants assemblies must be restrained from the hydrant back to the main.
- B. The Engineer may authorize hydrant protection using steel pipe bollards when hydrant installations have a greater than normal exposure to vehicular damage (e.g. parking lot installations, unusual driving situation, etc.). Install all such protection designated by the Engineer. Locate bollards as necessary adjacent to the hydrant and in such a manner as to not interfere with the ability to connect hoses or operate the hydrant as per detail drawing. Additionally, locate the bottom of the bollard and encasement above the hydrant supply piping and valve to prevent the possibility of damage to the piping should the bollard be displaced when hit. Payment for bollards shall be per the supplemental unit price schedule.
- C. Unless otherwise directed by the Engineer, excavate a drainage pit 2 feet in diameter and 2 feet deep below but not beyond each hydrant. Fill the pit with compacted ³/₄ inch clean granular under and around the base of the hydrant to a level 12 inches above the hydrant drain opening. No hydrant drainage pit shall be connected to a sewer.
- D. Cover the drainage area with geotextile fabric. The fabric shall completely isolate the gravel or stone so that no fill material or adjacent earth comes in contact with pit material.
- E. Notify the Engineer of situations where the ground water table is above the drain opening of dry barrel hydrants. If directed by Engineer, plug the drain opening

using a method acceptable to the hydrant manufacturer. No drainage pit is required when the hydrant drain is plugged. Mark the hydrant, in a manner acceptable to the Owner, to indicate that the drain opening has been plugged. Operation of a hydrant with plugged drain leaves the hydrant barrel full of water. Pump the hydrant barrel dry after each use.

F. Reaction or thrust blocking at the base of each hydrant must not obstruct the drainage outlet of the hydrant. The size and shape of concrete thrust backing and the number and size of tie rods, when required, shall be approved by the Engineer. Use the thrust blocking material specified in Specification Section 03300. See Specification Section 15000 for tie rod requirements.

3.03 TESTING

After installation and before backfilling (and after pressure testing the water main) test the hydrant as follows:

A. Pressure Test

- 1. Open the hydrant fully and fill with water; close all outlets.
- 2. To prevent caps from being blow off dry-barrel hydrants and to prevent other possible damage, vent air from the hydrant by leaving one of the caps slightly loose as the hydrant is being filled. After all air has escaped, tighten the cap before proceeding.
- 3. Apply line pressure.
- 4. Check for leakage at flanges, nozzles and operating stem.
- 5. If leakage is noted, repair or replace components or complete hydrant until no leaks are evident.

B. Drainage Test for Dry-Barrel Hydrants

- 1. Following the pressure test, close hydrant.
- 2. Remove one nozzle cap and place pylon or hand over nozzle opening.
- 3. Drainage rate should be sufficiently rapid to create a noticeable suction.
- 4. After backfilling, operate the hydrant to flush out any foreign material.
- 5. Tighten nozzle caps, them back them off slightly so that they will not be excessively tight; leave tight enough to prevent removal by hand.

- C. Paint all hydrant above the bury line in accordance with the local operations standards. Touch up paint (as specified by the Owner under Special Conditions) shall be applied upon completion of installation as needed. Take extreme care to avoid getting any paint on the "O" ring under the top operating nut or on the hydrant nozzles. Should paint be found on the "O" ring, the Contractor shall remove the paint and replace the "O" ring at their expense. Any paint on the hydrant nozzles shall be removed at the Contractor's expense.
 - 1 Alexandria City:
 - i. Public hydrants shall be painted with rust inhibitive primer and exterior enamel in the following color(s): Sherwin Williams "Safety Yellow" #B54YZ437 for barrels and Sherwin Williams "Pure White" #B54WZ401 for hydrant bonnets and caps. Exception: Public hydrant barrels may be painted with an approved flat black paint where such locations are specifically approved in writing by the fire chief.
 - Private hydrant shall be painted with a rust inhibitive primer and exterior enamel Sherwin Williams "Safety Yellow" #B54YZ437 for the barrels and bonnets and Sherman Williams "Pure White" #B54WZ401 for the caps only. Exception: Hydrant barrels may be painted with an approved flat black where such locations are specifically approved in writing by the fire chief.

AIR RELEASE AND BLOW-OFF OUTLETS (Contractor Furnished)

PART 1: GENERAL

1.01 SCOPE

The Contractor shall furnish and install air release and blow-off outlets at the locations shown on the Drawings or as directed by the Engineer. The details of the outlets shall be constructed as shown on Standard Details 0201-0601-SD4 and 0201-0601-SD64.

1.02 SUBMITTALS

Shop drawings and manufacturer's literature for equipment to be supplied shall be submitted to the Engineer for approval in accordance with Section 01300.

1.03 RELATED WORK

Section 03450 – Precast Concrete Manhole Section 15000 – Piping - General Provisions

PART 2: PRODUCTS

2.01 COMBINATION AIR/VACUUM RELEASE VALVES

- A. Bodies and Covers: Shall be of cast iron (ASTM A126, Class B, or ASTM A48, Class 35) or ductile iron (ASTM A536, Grade 65-45-12). Cover Bolts and nuts shall be stainless steel.
- B. Valve Connections: Flanged-end dimensions and drilling for cast-iron bodies and covers shall conform with ASME B16.1, Class 125 or Class 250. Flanged-end dimensions and drilling for ductile-iron bodies and covers shall conform with ASME B16.42, Class 150 or Class 300. Flanges shall be flat-faced unless otherwise specified by the Owner. Threaded-end connections shall conform with the requirements for tapered pipe threads for general use, per ASME B1.20.1.
- C. Floats: Float balls and guides shall be stainless steel. For valves with inlet sizes less than 4 inches, the float shall be capable of withstanding a collapse pressure of 1,000 psig. For valves with inlet sizes 4 inches and larger, the float shall be capable of withstanding collapse pressures of 750 psig.
- D. Venting: Air release valves and the air release mechanism of combination valves shall be designed to open positively and vent air to the atmosphere at system pressures up to the maximum working pressure. Orifices shall be sized accordingly. The vent pipe shall be continuous from the valve to 2 feet (minimum) above finished grade and shall be provided with a #14 mesh screened, downward-facing elbow.

- E. For sewage force main applications, provide special long-body sewage air release valves, complete with top and bottom flushing connections and hose(s).
- F. The Contractor shall provide APCO Model No. 145C as manufactured by Valve and Primer Corporation, Schaumburg, Illinois. Bodies shall be case iron with stainless steel floats.

PART 3: EXECUTION

3.01 INSTALLATION

- A. Valves shall be Installed in accordance with the requirements contained in local State and City/County water works regulations, Section 15000, and Drawings.
- B. If required by AW Project Manager, provide services of technical representative of valve manufacturer available on-site during installation of valves.
- C. Prior to installation, remove foreign matter from within valves. Inspect valves in open and closed position to verify that the parts are in satisfactory working condition.
- D. Install valves and valve manholes and vaults where indicated on Drawings or as located by the AW Project Manager. Set manholes and vaults plumb and as detailed. Center manholes on valves. Compact around each manhole and vault for a minimum radius of 4 feet, or to undisturbed trench face, when less than 4 feet. Provide above-ground vents for manholes and vaults as indicted on Drawings.

3.02 DISINFECTION AND TESTING

- A. Disinfect water lines, valves, and appurtenances as required by Section 15020 or 15021, as applicable.
- B. Conduct pressure and leakage tests as required by Section 02457.

3.03 DISINFECTION AND TESTING

A. Paint piping and valves located in vaults, stations, and above ground.

SERVICE LINES (Contractor Furnished)

PART 1: GENERAL

1.01 SCOPE1

Furnish and install service lines originating at the water main and terminating at a curb stop connection where shown on the Drawings or described in the Specification Special Conditions. This Specification Section does not include service lines or meter installations beyond the curb stop. Refer to Standard Details for a typical service line installation.

1.02 RELATED WORK

Section 15000 – Piping - General Provisions

1.03 REFERENCES

Refer to current AWWA Standards: AWWA Standard for Underground Service Valves and Fittings C800.

PART 2: PRODUCTS

All Products described below shall meet the requirements of NSF 61.

Research has documented that certain pipe materials (such as polyethylene) and certain elastomers (such as those used in gasket material and packing glands) may be subject to permeation by lower-molecular weight organic solvents or petroleum products. Products supplied under this Specification Section assume that petroleum products or organic solvents will not be encountered. If during the course of pipeline installation, the Contractor identifies, or suspects the presence of petroleum products or any unknown chemical substance, notify the Engineer immediately. Stop installing piping in the area of suspected contamination until direction is provided by the Engineer.

All connections to copper pipe shall utilize flared or compression connection, as directed by Owner engineer.

2.01 COPPER SERVICE LINE MATERIAL

Copper pipe shall be Type K, as specified, meeting the requirements of ASTM Standard B88. The pipe size $(3/4, 1, 1 \frac{1}{2}, \text{ or } 2 \text{ inch})$ are to be determined by the Engineer.

2.02 POLYETHYLENE SERVICE LINE MATERIAL

Polyethylene service line material shall be Class 160 (minimum), ultra-high molecular weight, conforming to AWWA Standard C901. Pipe sizes (3/4, 1, 1 ½, or 2 inch), copper tube size (CTS) or iron pipe size (IPS) to be determined by the Engineer.

Acceptable manufacturers:

- A. Endot Industries EndoPure PE-3408 only
- B. J-M Manufacturing
- C. KWH Pipe

2.03 CORPORATION STOPS

Corporation stops shall be of the brass, ball valve type manufactured in accordance with AWWA Standard C800. The inlet connection shall have standard AWWA tapered threads unless otherwise required by the Engineer. The outlet connection shall be copper or brass compression or flare connection end or pack joint for polyethylene pipe, as required. Dielectric unions shall be used to prevent transfer of any electrical stray currents from metallic service lines to metallic water main. The sizes shall range from $\frac{1}{2}$ inch to 2 inch and shall match the size of specified service line material.

Acceptable manufacturers and model numbers are:

- A. Ford Meter Box Company FB400 thru FB1600
- B. Mueller B-25000
- C. A.Y. McDonald 4701B Series

2.04 CURB STOPS

Curb stops shall be bronze body construction, ball valves, with Double O-ring stem seals. Curb stops shall conform to AWWA Standard C800. End connections shall be suitable for copper or brass compression connection or pack joint for polyethylene pipe, as required. Sizes shall be from ³/₄ inch to 2 inch and shall match the service line size.

Acceptable manufacturers and model numbers:

- A. Ford Meter Box Company B22 Series
- B. Mueller B-25204
- C. A.Y. McDonald 6100 Series

2.05 CURB BOXES

Curb boxes shall be standard cast iron, sliding or screw type, 1 inch or 2 $\frac{1}{2}$ inch as required, complete with lid and head bolt. Boxes shall be adjustable from 18-inches to 66-inches. The box size will be determined by the Engineer.

Acceptable manufacturers:

- A. Bingham & Taylor
- B. Mueller
- C. Handley Industries
- D. Clay & Bailey
- E. A.Y. McDonald Quality Water Products

2.06 MISCELLANEOUS SERVICE LINE FITTINGS

Miscellaneous service line fittings such as couplings, adapters, saddles, bends, plugs, service line electrical insulators, etc. shall conform to AWWA Standard C800.

Acceptable manufacturers:

- A. Ford Meter Box
- B. Mueller
- C. A.Y. McDonald

PART 3: EXECUTION

3.01 INSTALLATION OF CORPORATION STOPS

- A. Use experienced craftsmen familiar with installation of water service lines when tapping water mains. Make all taps with a suitable tapping machine (Mueller, Ford, Hays or Dresser type) using the proper combined drill and tap. Hand-held drilling equipment is not acceptable.
- B. Before making the tap, inspect corporation stops for cleanliness, damaged threads, and proper operation of the ball valve prior to installation. Do not install corporation stops that fail this inspection.
- C. The main may be tapped along the top half of the pipe as directed by the Engineer or as shown on Standard Details. Use a tapping saddle when the water main wall thickness or material (plastic, concrete or A-C pipeline material) make it unsuitable for direct tapping. Verify saddle use with Engineer.
- D. In the case of multiple services of small diameter (less than 2-inch diameter), corporation stops shall be at least 12 inches apart and at least 22-1/2 degrees above or below the location of any adjacent tap(s) and curb stops and boxes shall be at least one foot apart. In the case of large diameter multiple services, tap at least 24 inches apart and at least 22-1/2 degrees above or below the location of any adjacent tap(s).
- E. Install all corporation stops so that between 2 and 3 threads extend beyond the inside wall of the main. If necessary, make a test tap with the boring bar marked to the proper depth. The corporation stop, when properly installed, will not be shouldered with the main. Do not use lubricants of any type when installing the corporation stop.
- F. Use the procedures outlined in AWWA Standard C600 for installing taps on grey iron or ductile iron mains encased in polyethylene.

3.02 INSTALLATION OF SERVICE LINE AND FITTINGS

- A. Excavate the service line trench in accordance with Division 2 of these Specifications. Where augering or moling is permitted, follow guidelines provided by the equipment manufacturer including making a proper size hole to launch and receive the unit. If moling or augering is employed, take appropriate precautions to avoid damaging other utilities and disturbing the unexcavated surface.
- B. Install service line between the tap connection and the curb stop location making only gradual changes in grade or alignment as required. Sharp bends (greater than 15 degrees) in any direction are not allowed unless approved by the Engineer. 1 ½-inch and 2-inch service lines may be installed using three (3) 1-inch corporation stops and a 3-branch connection. This is in lieu of installing a 1 ½-inch or 2-inch corporation stop. Installation shall be in accordance with Specification Section 15000 and Standard Details and in accordance with local regulators.
- C. Install all services straight and at right angles to the main. If this cannot be accomplished, provide the Owner with accurate as-built dimensions to the tee or corporation stop. The Contractor may be required to attach Owner supplied magnets to curb box and valve box.
- D. All trench services shall be installed with marking tape. This tape shall provide an early warning at shallow depth excavation. The non-detectable tape shall be 6 inches wide and buried approximately 12 inches above the service pipe, but a minimum of 12 inches below finished grade. It shall consist of multiple layers of polyethylene with an overall thickness of 3 to 5 mils. The black colored lettering on the warning tape shall be abrasion resistant and be imprinted on a color-coded background that conforms to APWA color code standards. It shall be installed continuous from the corporation stop to the curb stop.
- E. All plastic service line connections shall use insert stiffeners of the appropriate length and size.

3.03 INSTALLATION OF CURB STOPS

- A. Install curb stops with the operating nut in the vertical position and the curb box centered over the nut. Install curb boxes plum and adjusted to be flush with finished grade. Install and lock curb boxes immediately after installation.
- B. After completion of service line installation, but prior to backfilling, open the corporation stop slowly to fill the line. When the line is full and all air has been removed, completely open the corporation and close the curb stop. Visually inspect that all piping, fittings, and taps for leaks. Backfill and restore the surface the service line trench in accordance with Division 2 of these Specifications.

3.04 POLYETHYLENE ENCASEMENT

Provide polyethylene encasement, or other protective wrap approved by the Engineer, on all metal service lines and fittings (pile, valves, stops, etc.) when they are made of different materials than the water main. When the polyethylene is applied on the main, it shall extend for a minimum clear distance of three (3) feet away from the main when services are not being renewed or extend from the main connection to and including the curb stop or curb meter setter for all new copper service lines. Encasement material and installation shall be per Specification Section 15130 or 15131 and AWWA Standard C105.